APPENDIX G2

PRELIMINARY HYDROLOGY REPORT

PRELIMINARY HYDROLOGY REPORT

FOR

VALLEY GARDENS

13989 MORENO ROSE PLACE

MORENO VALLEY, CA 92553

PEN21-0250 (LST21-0073)

PREPARED FOR:

MORENO VALLEY GARDEN, LLC

39903 CAMDEN COURT

TEMECULA, CA 92591

PREPARED BY:

WABER CONSULTANTS, INC.

19210 S VERMONT AVE., SUITE 115

GARDENA, CA 90248

(424) 344-2464

OCTOBER 2022

REGISTERED PROFESSIONAL ENGINEER
MAHIR WABER
No. C69050
Exp. 6/30/24
CIVIL
STATE OF CALIFORNIA

Mahir Waber

Table of Contents

Tables

Appendices

1.0 Scope

Hydrologic calculations to evaluate surface runoff associated with 2-year, 5-year, 10-year, 25-year, 50-year and 100-year hypothetical design storm frequency from the tributary drainage areas were performed based on the latest *Riverside County Flood Control and Water Conservation District* rational method. Hydrologic parameters used in the analysis, such as rainfall and soil classification are presented in the *Riverside County Hydrology Manual* (Hydrology Manual).

2.0 Project Description

2.1. Existing Conditions

The subject property is located along the north side of Alessandro Boulevard, east of Flaming Arrow Drive, in Moreno Valley, California. The site is a vacant lot covered with light vegetation approximately 4.85 acres. It is bounded by single home residences to the north and west, Sarah Street to the east and Alessandro Boulevard to the south. The existing site is relatively level with topography descending gradually from north to south on the order of a few feet.

2.2. Proposed Conditions

The proposed project will include 8 residential rental apartment buildings, 1 office building and a surface parking lot area.

Per soil report by NorCal Engineering dated August 27,2021, site has underlying soil with low infiltration rates. Therefore, the site is proposing 16 vegetated bioretention areas, 1 modular wetland system and 1 sump pump on the site.

3.0 Hydrology

3.1 Methodology

The hydrologic calculations to determine the 2-year, 5-year, 10-year, 25-year, 50-year and 100-year peak flow rates were performed using the criteria in the *Riverside County Flood Control District and Riverside County Hydrology Manual*. The Rational Method is an empirical computation procedure for developing a peak runoff rate (discharge) for storms of a specific recurrence interval. Rational Method equations are based on the assumption that the peak flow rate is directly proportional to the drainage area, rainfall intensity, and a loss rate coefficient, which describes the effects of land use and soil type. The Rational Method flow rates were computed by generating a hydrologic "link-node" model, which divides the area into drainage subareas. Please see Appendix A for hydrology calculations.

3.2 Areas

Hydrology Maps are included in Appendix C of this report delineating the drainage subareas. Areas are provided in the maps in both square feet (SF) and acres (AC). AC units are used in the rational method calculations. Hydrology Maps are provided in Appendix C of this report.

3.3 Soil

When making estimates of storm water runoff it is assumed that infiltration is a loss for the storm event under consideration. The major affecting infiltration is the nature of the soil itself. The site is underlain by soil with slow infiltration rates. Therefore, Soil Type C was selected for the hydrology analysis.

3.4 Time of Concentration

The Time of Concentration (T_c) is the time required for runoff to flow from the most remote part of the drainage area to the point of interest. The T_c (minutes) is based on slope and runoff coefficient, and it was obtained using the nomograph in Plate D-3 of the Hydrology Manual, and it is included in Appendix B of this report for reference.

3.5 Rainfall Intensity

The rainfall intensity is the rainfall in inches per hour (in/hr) for a duration equal to the T_c for a selected storm frequency. Intensity is dependent on precipitation and T_c. The time-averaged rainfall intensity for the 2-year, 5-year, 10-year, 25-year, 50-year and 100-year storm event were obtained from the precipitation intensity curves using the regression equation in Plate D-4.7 of the Hydrology Manual. The regression equations determine the precipitation intensities corresponding to the time of concentrations and selected design frequency. Calculations of intensities are provided as part of the hydrology calculations and included in Appendix A.

3.6 Hydrology

The peak rate runoff flow of the proposed site increases due to increase in impervious areas including roofs, drive aisles, and sidewalks. The existing and proposed flows were calculated using the Rational Method based on the site conditions discussed in Sections 2.1 and 2.2, respectively.

3.6.1 Existing Hydrology

The entire existing site sheet flows in a generally southerly direction towards the south side of the property. Runoff from the site eventually sheet flows onto Alessandro Boulevard to the south of the property into an existing catch basin just west of Sarah Street. There is no offsite flow towards the property. There is a block wall on the north and west side of the property. The residential building located east of the property has flow draining south towards Alessandro Blvd. The existing flow for the different storm frequencies is outlined in Table 1 below.

Table 1 - Summary of Existing Flows

Subarea	Area							
	2-year	5-year	10-year	25-year	50-year	100-year	(sf)	(ac)
Area 1	2.23	3.36	4.23	5.26	6.47	7.34	211,195	4.85
Total	**2.23**	**3.36**	**4.23**	**5.26**	**6.47**	**7.34**	**211,195**	**4.85**

3.6.2 Proposed Hydrology

The proposed project site has been subdivided into subareas for runoff of storm water based on drainage patterns including ridge lines and low/confluence points. The drainage patterns include the roof surface runoff and ground surface runoff areas. There is no offsite flow towards the property. There is a block wall on the north and west side of the property. The residential building located east of the property has flow draining south towards Alessandro Blvd. Areas 1 to 16 drain towards bioretention basin and area 17 drain to MWS. The bioretention basins and MWS flow towards a sump pump that is connected to a catch basin on Alessandro Blvd. Each subarea and the discharge point of each subarea is identified in the Proposed Hydrology Map. Flow for each subarea is outlined in Table 2 below:

Table 2 - Summary of Proposed Flows

Subarea							Area	
	2-year	5-year	10-year	25-year	50-year	100-year	(sf)	(ac)
Area 1	0.22	0.29	0.37	0.43	0.52	0.58	7,132	0.16
Area 2	0.50	0.66	0.83	0.97	1.18	1.31	16,281	0.37
Area 3	0.26	0.34	0.43	0.50	0.61	0.63	8,349	0.19
Area 4	0.53	0.72	0.87	1.04	1.22	1.36	20,137	0.46
Area 5	1.12	1.55	1.85	2.21	2.63	2.96	52,491	1.21
Area 6	0.19	0.25	0.32	0.37	0.45	0.50	6,208	0.14
Area 7	0.14	0.19	0.23	0.27	0.33	0.37	4,622	0.11
Area 8	0.40	0.53	0.66	0.77	0.93	1.04	13,004	0.40
Area 9	0.50	0.66	0.83	0.96	1.17	1.30	16,061	0.50
Area 10	0.19	0.25	0.32	0.37	0.45	0.50	6,166	0.19
Area 11	0.14	0.18	0.23	0.27	0.33	0.34	4,491	0.14
Area 12	0.30	0.39	0.49	0.57	0.70	0.78	9,569	0.30
Area 13	0.42	0.55	0.69	0.81	0.98	1.10	13,708	0.42
Area 14	0.16	0.22	0.26	0.32	0.37	0.42	6,330	0.16
Area 15	0.26	0.36	0.42	0.51	0.60	0.67	10,244	0.26
Area 16	0.16	0.22	0.26	0.31	0.37	0.41	5,726	0.16
Area 17	0.26	0.33	0.39	0.49	0.55	0.62	10,676	0.26
Total	**5.74**	**7.70**	**9.45**	**11.16**	**13.38**	**14.88**	**211,195**	**4.85**

4.0 Conclusion

The overall drainage patterns in the proposed condition are similar to the existing condition in terms of the overall drainage direction. However, the proposed drainage patterns are divided into subareas as shown on the attached Hydrology Map – Proposed Condition. The subareas account for the ridges in the roof areas as well as the ground surfaces including the drive aisles, parking spaces, and landscape areas.

Due to increase in impervious areas, the proposed site generates more flow than the existing condition. Table 3 below summarizes the flows of the existing and proposed site.

Table 3 - Pre- and Post-Construction Flows

Storm Event	Existing Q (CFS)	Proposed Q (CFS)
2-yr	2.23	5.74
5-yr	3.36	7.70
10-yr	4.23	9.45
25-yr	5.26	11.16
50-yr	6.47	13.38
100-yr	7.34	14.88

This site's runoff is mitigated by proposing a storm drain system that includes 16 vegetated bioretention basins (Areas 1 to 16) and MWS (Area 17). The vegetated basins and MWS are connected to a sump pump that flows towards a catch basin on Alessandro Blvd. A summary of the required and mitigated volume for each area is provided in Table 4 below. Calculations for required and mitigated volume for each area is prepared for Compliance with Reginal Board No. R8-2010-0033 as shown in WQMP report.

Table 4 - Summary of Required and Proposed Volume

Sub-area	Required V (cu-ft)	Proposed V (cu-ft)
Area - 1	125.2	140
Area - 2	268.2	294
Area - 3	144.1	171.5
Area - 4	795.9	799
Area - 5	1,885.2	2,025
Area - 6	135.8	141.75
Area – 7	136	141
Area – 8	254.3	262
Area – 9	274.6	278
Area - 10	124.2	141.75
Area – 11	115.2	115.3
Area – 12	170.3	182
Area - 13	285.1	336
Area – 14	206.5	206.5
Area – 15	416.9	414
Area - 16	159.4	169
Area - 17	531.7	532

Appendix A – Hydrology Calculations

HYDROLOGY CALCULATIONS
VALLEY GARDENS

EXISTING AREA

Q = CIA

A = 211,195 sf = 4.85 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 18.5 min (Plate D-3)

T_c = duration, min

I_2 = 0.88 in/hr (Plate D-4.7)

C_2 = 0.52 in/hr (Plate D-5.3)

Q_2 = 2.23 cfs

I_5 = 1.18 in/hr (Plate D-4.7)

C_5 = 0.59 in/hr (Plate D-5.3)

Q_5 = 3.36 cfs

I_{10} = 1.40 in/hr (Plate D-4.7)

C_{10} = 0.62 in/hr (Plate D-5.3)

Q_{10} = 4.23 cfs

I_{25} = 1.65 in/hr (Plate D-4.7)

C_{25} = 0.66 in/hr (Plate D-5.3)

Q_{25} = 5.26 cfs

I_{50} = 1.94 in/hr (Plate D-4.7)

C_{50} = 0.69 in/hr (Plate D-5.3)

Q_{50} = 6.47 cfs

I_{100} = 2.15 in/hr (Plate D-4.7)

C_{100} = 0.70 in/hr (Plate D-5.3)

Q_{100} = 7.34 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-1

Q = CIA

A = 7,132 sf = 0.16 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.3 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.22 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.29 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.37 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.85 in/hr (Plate D-5.3)

Q_{25} = 0.43 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.86 in/hr (Plate D-5.3)

Q_{50} = 0.52 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 0.58 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-2

Q = CIA Where Q = proposed peak flows, cfs

A = total area, acres

A = 16,281 sf = 0.37 acres C = coefficient of runoff

I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.1 min (Plate D-3) T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.50 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.66 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.83 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.97 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 1.18 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 1.31 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-3

Q = CIA

A = 8,349 sf = 0.19 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.1 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.26 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.34 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.43 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.85 in/hr (Plate D-5.3)

Q_{25} = 0.50 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.86 in/hr (Plate D-5.3)

Q_{50} = 0.61 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.80 in/hr (Plate D-5.3)

Q_{100} = 0.63 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-4

Q = CIA

A = 20,137 sf = 0.46 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 7.3 min (Plate D-3)

T_c = duration, min

I_2 = 1.40 in/hr (Plate D-4.7)

C_2 = 0.82 in/hr (Plate D-5.3)

Q_2 = 0.53 cfs

I_5 = 1.88 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.72 cfs

I_{10} = 2.22 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.87 cfs

I_{25} = 2.64 in/hr (Plate D-4.7)

C_{25} = 0.85 in/hr (Plate D-5.3)

Q_{25} = 1.04 cfs

I_{50} = 3.08 in/hr (Plate D-4.7)

C_{50} = 0.86 in/hr (Plate D-5.3)

Q_{50} = 1.22 cfs

I_{100} = 3.42 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 1.36 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-5

Q = CIA

A = 52,491 sf = 1.21 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 10.4 min (Plate D-3)

T_c = duration, min

I_2 = 1.17 in/hr (Plate D-4.7)

C_2 = 0.79 in/hr (Plate D-5.3)

Q_2 = 1.12 cfs

I_5 = 1.58 in/hr (Plate D-4.7)

C_5 = 0.81 in/hr (Plate D-5.3)

Q_5 = 1.55 cfs

I_{10} = 1.86 in/hr (Plate D-4.7)

C_{10} = 0.82 in/hr (Plate D-5.3)

Q_{10} = 1.85 cfs

I_{25} = 2.20 in/hr (Plate D-4.7)

C_{25} = 0.83 in/hr (Plate D-5.3)

Q_{25} = 2.21 cfs

I_{50} = 2.59 in/hr (Plate D-4.7)

C_{50} = 0.84 in/hr (Plate D-5.3)

Q_{50} = 2.63 cfs

I_{100} = 2.89 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 2.96 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-6

Q = CIA

A = 6,208 sf = 0.14 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.7 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.19 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.25 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.32 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.37 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.45 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 0.50 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-7

Q = CIA

A = 4,622 sf = 0.11 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.7 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.80 in/hr (Plate D-5.3)

Q_2 = 0.14 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.82 in/hr (Plate D-5.3)

Q_5 = 0.19 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.83 in/hr (Plate D-5.3)

Q_{10} = 0.23 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.27 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.33 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 0.37 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-8

Q = CIA

A = 13,004 sf = 0.30 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.2 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.80 in/hr (Plate D-5.3)

Q_2 = 0.40 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.82 in/hr (Plate D-5.3)

Q_5 = 0.53 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.83 in/hr (Plate D-5.3)

Q_{10} = 0.66 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.77 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.93 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 1.04 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-9

$Q = CIA$

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

A = 16,061 sf = 0.37 acres

Soil Group = C (Plate C-1.17)

T_c = 4.4 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.50 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.66 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.83 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.85 in/hr (Plate D-5.3)

Q_{25} = 0.96 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.86 in/hr (Plate D-5.3)

Q_{50} = 1.17 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 1.30 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-10

Q = CIA

A = 6,166 sf = 0.14 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.5 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.19 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.25 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.32 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.37 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.45 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 0.50 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-11

Q = CIA

A = 4,491 sf = 0.10 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.5 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.14 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.18 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.23 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.85 in/hr (Plate D-5.3)

Q_{25} = 0.27 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.86 in/hr (Plate D-5.3)

Q_{50} = 0.33 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.80 in/hr (Plate D-5.3)

Q_{100} = 0.34 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-12

Q = CIA

A = 9,569 sf = 0.22 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.5 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.82 in/hr (Plate D-5.3)

Q_2 = 0.30 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.39 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.49 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.85 in/hr (Plate D-5.3)

Q_{25} = 0.57 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.86 in/hr (Plate D-5.3)

Q_{50} = 0.70 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 0.78 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-13

Q = CIA

A = 13,708 sf = 0.31 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 4.4 min (Plate D-3)

T_c = duration, min

I_2 = 1.67 in/hr (Plate D-4.7)

C_2 = 0.79 in/hr (Plate D-5.3)

Q_2 = 0.42 cfs

I_5 = 2.15 in/hr (Plate D-4.7)

C_5 = 0.81 in/hr (Plate D-5.3)

Q_5 = 0.55 cfs

I_{10} = 2.67 in/hr (Plate D-4.7)

C_{10} = 0.82 in/hr (Plate D-5.3)

Q_{10} = 0.69 cfs

I_{25} = 3.07 in/hr (Plate D-4.7)

C_{25} = 0.83 in/hr (Plate D-5.3)

Q_{25} = 0.81 cfs

I_{50} = 3.70 in/hr (Plate D-4.7)

C_{50} = 0.84 in/hr (Plate D-5.3)

Q_{50} = 0.98 cfs

I_{100} = 4.10 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 1.10 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-14

Q = CIA

A = 6,330 sf = 0.15 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 5.1 min (Plate D-3) T_c = duration, min

I_2 = 1.36 in/hr (Plate D-4.7)

C_2 = 0.81 in/hr (Plate D-5.3)

Q_2 = 0.16 cfs

I_5 = 1.85 in/hr (Plate D-4.7)

C_5 = 0.83 in/hr (Plate D-5.3)

Q_5 = 0.22 cfs

I_{10} = 2.17 in/hr (Plate D-4.7)

C_{10} = 0.84 in/hr (Plate D-5.3)

Q_{10} = 0.26 cfs

I_{25} = 2.58 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.32 cfs

I_{50} = 3.00 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.37 cfs

I_{100} = 3.35 in/hr (Plate D-4.7)

C_{100} = 0.86 in/hr (Plate D-5.3)

Q_{100} = 0.42 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-15

Q = CIA

A = 10,244 sf = 0.24 acres

Where

Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 7.6 min (Plate D-3)

T_c = duration, min

I_2 = 1.36 in/hr (Plate D-4.7)

C_2 = 0.80 in/hr (Plate D-5.3)

Q_2 = 0.26 cfs

I_5 = 1.85 in/hr (Plate D-4.7)

C_5 = 0.82 in/hr (Plate D-5.3)

Q_5 = 0.36 cfs

I_{10} = 2.17 in/hr (Plate D-4.7)

C_{10} = 0.83 in/hr (Plate D-5.3)

Q_{10} = 0.42 cfs

I_{25} = 2.58 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.51 cfs

I_{50} = 3.00 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.60 cfs

I_{100} = 3.35 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 0.67 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-16

Q = CIA

A = 5,726 sf = 0.13 acres

Where
Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 6.2 min (Plate D-3)

T_c = duration, min

I_2 = 1.51 in/hr (Plate D-4.7)

C_2 = 0.80 in/hr (Plate D-5.3)

Q_2 = 0.16 cfs

I_5 = 2.04 in/hr (Plate D-4.7)

C_5 = 0.82 in/hr (Plate D-5.3)

Q_5 = 0.22 cfs

I_{10} = 2.41 in/hr (Plate D-4.7)

C_{10} = 0.83 in/hr (Plate D-5.3)

Q_{10} = 0.26 cfs

I_{25} = 2.85 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.31 cfs

I_{50} = 3.32 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.37 cfs

I_{100} = 3.71 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 0.41 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

AREA-17

Q = CIA

A = 10,676 sf = 0.25 acres

Where Q = proposed peak flows, cfs
A = total area, acres
C = coefficient of runoff
I = rainfall intensity (in/hr) corresponding to the time of concentration

Soil Group = C (Plate C-1.17)

T_c = 9.7 min (Plate D-3)

T_c = duration, min

I_2 = 1.31 in/hr (Plate D-4.7)

C_2 = 0.80 in/hr (Plate D-5.3)

Q_2 = 0.26 cfs

I_5 = 1.64 in/hr (Plate D-4.7)

C_5 = 0.82 in/hr (Plate D-5.3)

Q_5 = 0.33 cfs

I_{10} = 1.92 in/hr (Plate D-4.7)

C_{10} = 0.83 in/hr (Plate D-5.3)

Q_{10} = 0.39 cfs

I_{25} = 2.36 in/hr (Plate D-4.7)

C_{25} = 0.84 in/hr (Plate D-5.3)

Q_{25} = 0.49 cfs

I_{50} = 2.65 in/hr (Plate D-4.7)

C_{50} = 0.85 in/hr (Plate D-5.3)

Q_{50} = 0.55 cfs

I_{100} = 2.96 in/hr (Plate D-4.7)

C_{100} = 0.85 in/hr (Plate D-5.3)

Q_{100} = 0.62 cfs

HYDROLOGY CALCULATIONS
VALLEY GARDENS

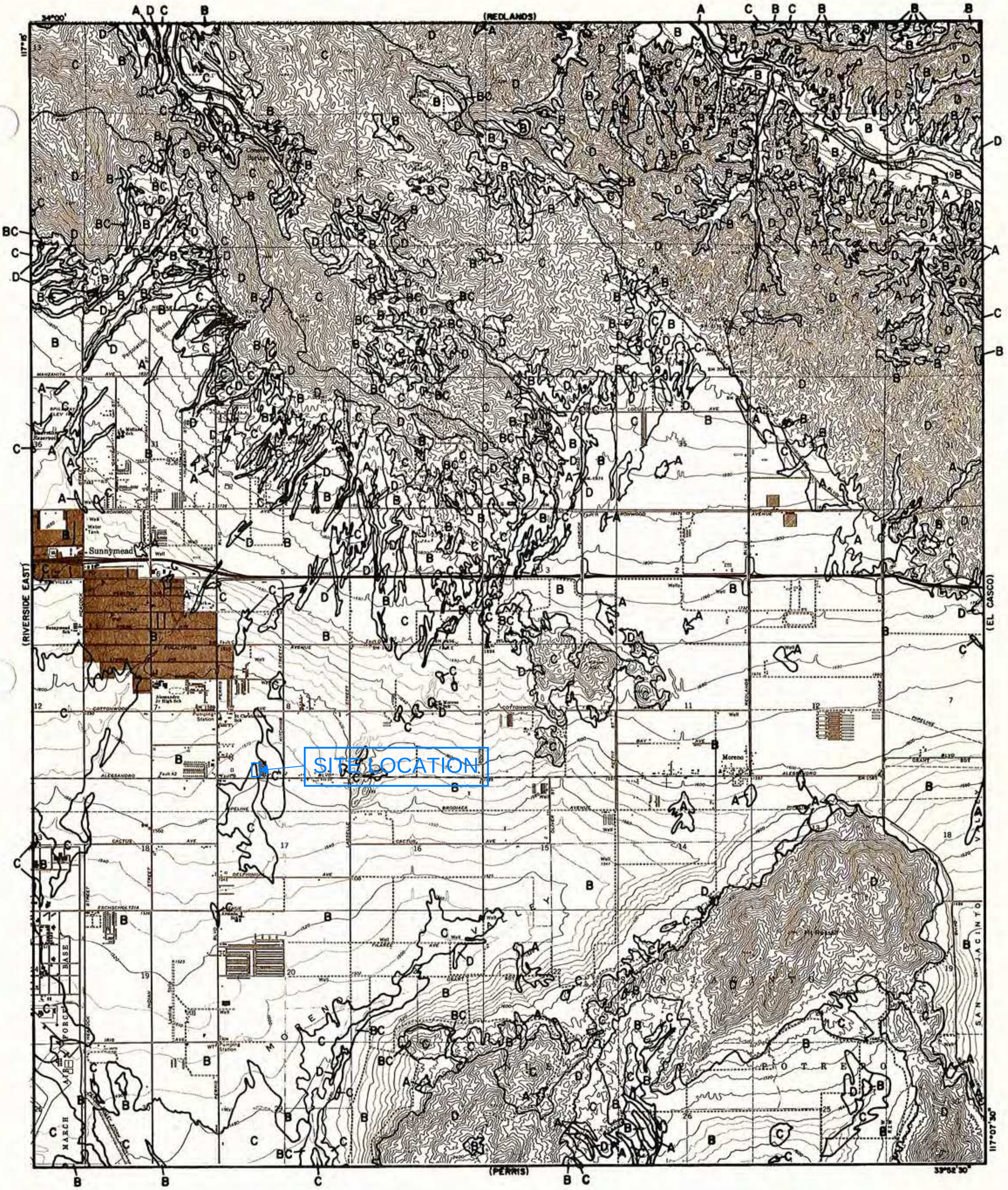
Existing

Area	Area							
	2-year	5-year	10-year	25-year	50-year	100-year	(sf)	(ac)
Area 1	2.23	3.36	4.23	5.26	6.47	7.34	211,195	4.85

Proposed

Subarea	Area							
	2-year	5-year	10-year	25-year	50-year	100-year	(sf)	(ac)
Area 1	0.22	0.29	0.37	0.43	0.52	0.58	7,132	0.16
Area 2	0.50	0.66	0.83	0.97	1.18	1.31	16,281	0.37
Area 3	0.26	0.34	0.43	0.50	0.61	0.63	8,349	0.19
Area 4	0.53	0.72	0.87	1.04	1.22	1.36	20,137	0.46
Area 5	1.12	1.55	1.85	2.21	2.63	2.96	52,491	1.21
Area 6	0.19	0.25	0.32	0.37	0.45	0.50	6,208	0.14
Area 7	0.14	0.19	0.23	0.27	0.33	0.37	4,622	0.11
Area 8	0.40	0.53	0.66	0.77	0.93	1.04	13,004	0.40
Area 9	0.50	0.66	0.83	0.96	1.17	1.30	16,061	0.50
Area 10	0.19	0.25	0.32	0.37	0.45	0.50	6,166	0.19
Area 11	0.14	0.18	0.23	0.27	0.33	0.34	4,491	0.14
Area 12	0.30	0.39	0.49	0.57	0.70	0.78	9,569	0.30
Area 13	0.42	0.55	0.69	0.81	0.98	1.10	13,708	0.42
Area 14	0.16	0.22	0.26	0.32	0.37	0.42	6,330	0.16
Area 15	0.26	0.36	0.42	0.51	0.60	0.67	10,244	0.26
Area 16	0.16	0.22	0.26	0.31	0.37	0.41	5,726	0.16
Area 17	0.26	0.33	0.39	0.49	0.55	0.62	10,676	0.26
Total	5.74	7.70	9.45	11.16	13.38	14.88	211,195	4.85

Appendix B – Reference Figures and Tables



LEGEND

—— SOILS GROUP BOUNDARY

A SOILS GROUP DESIGNATION

RCFC & WCD

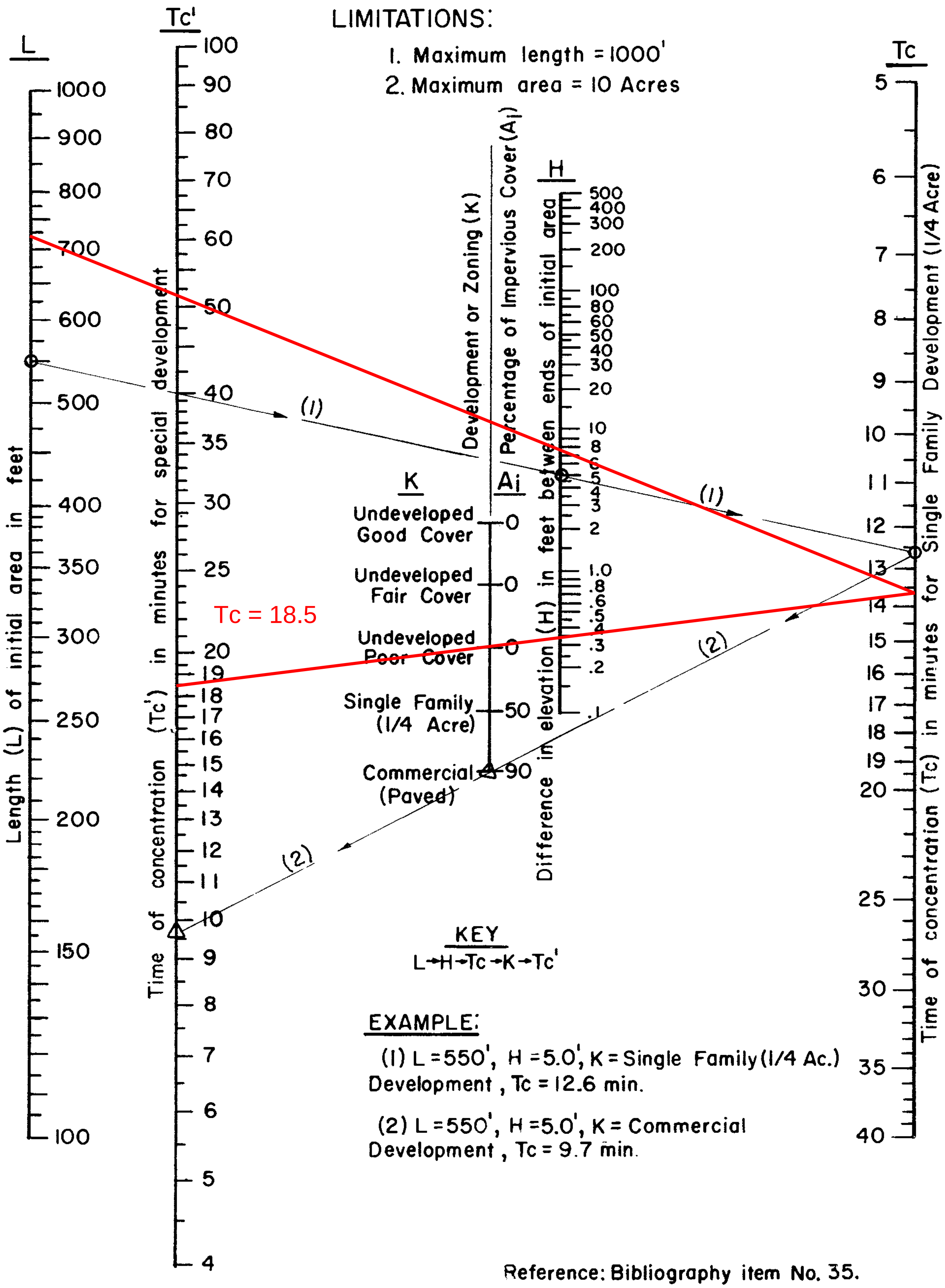
HYDROLOGY MANUAL

0 FEET 5000

HYDROLOGIC SOILS GROUP MAP FOR SUNNYMEAD

PLATE C-1.17

EXISTING AREA



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

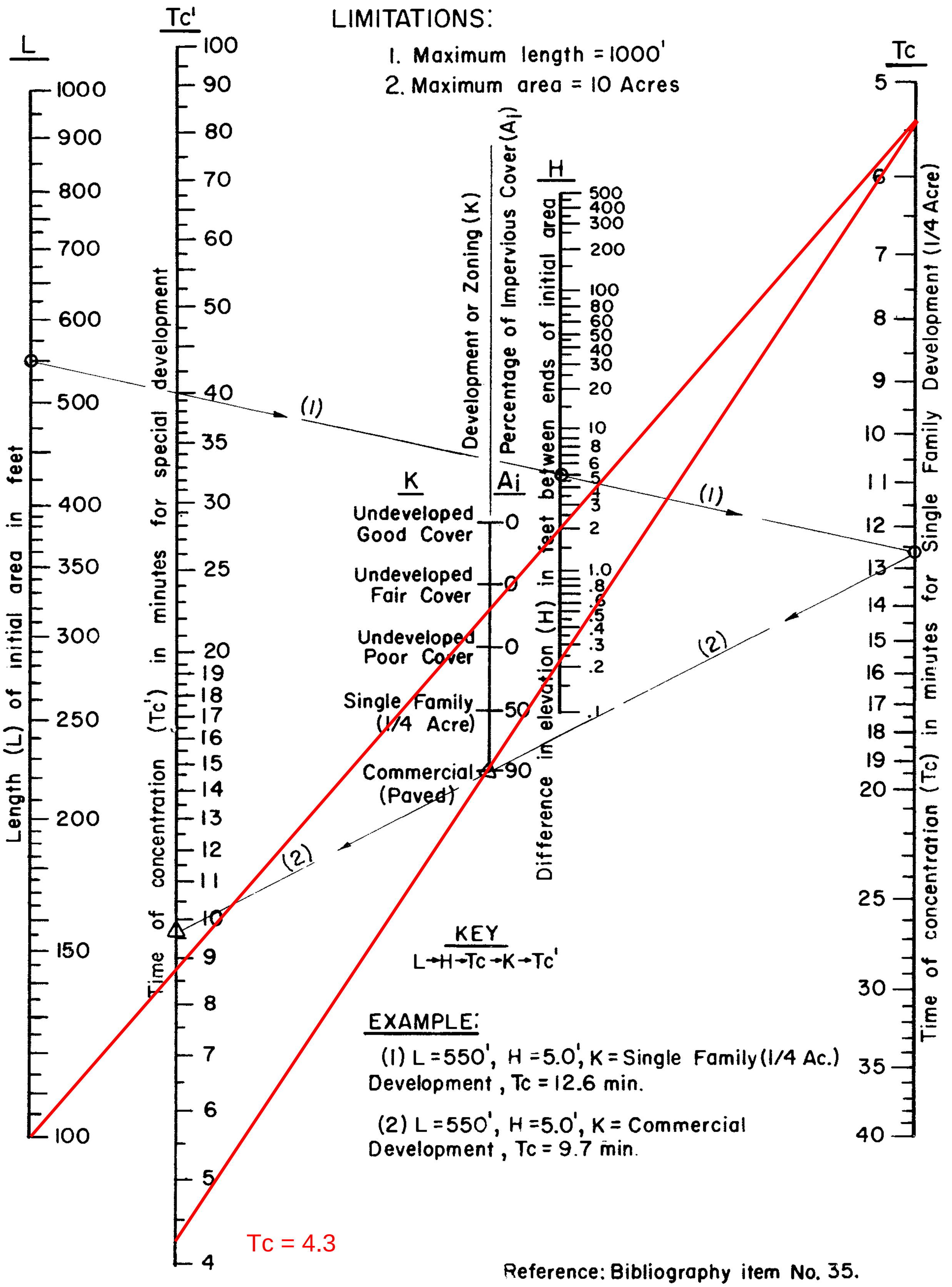
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 1



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

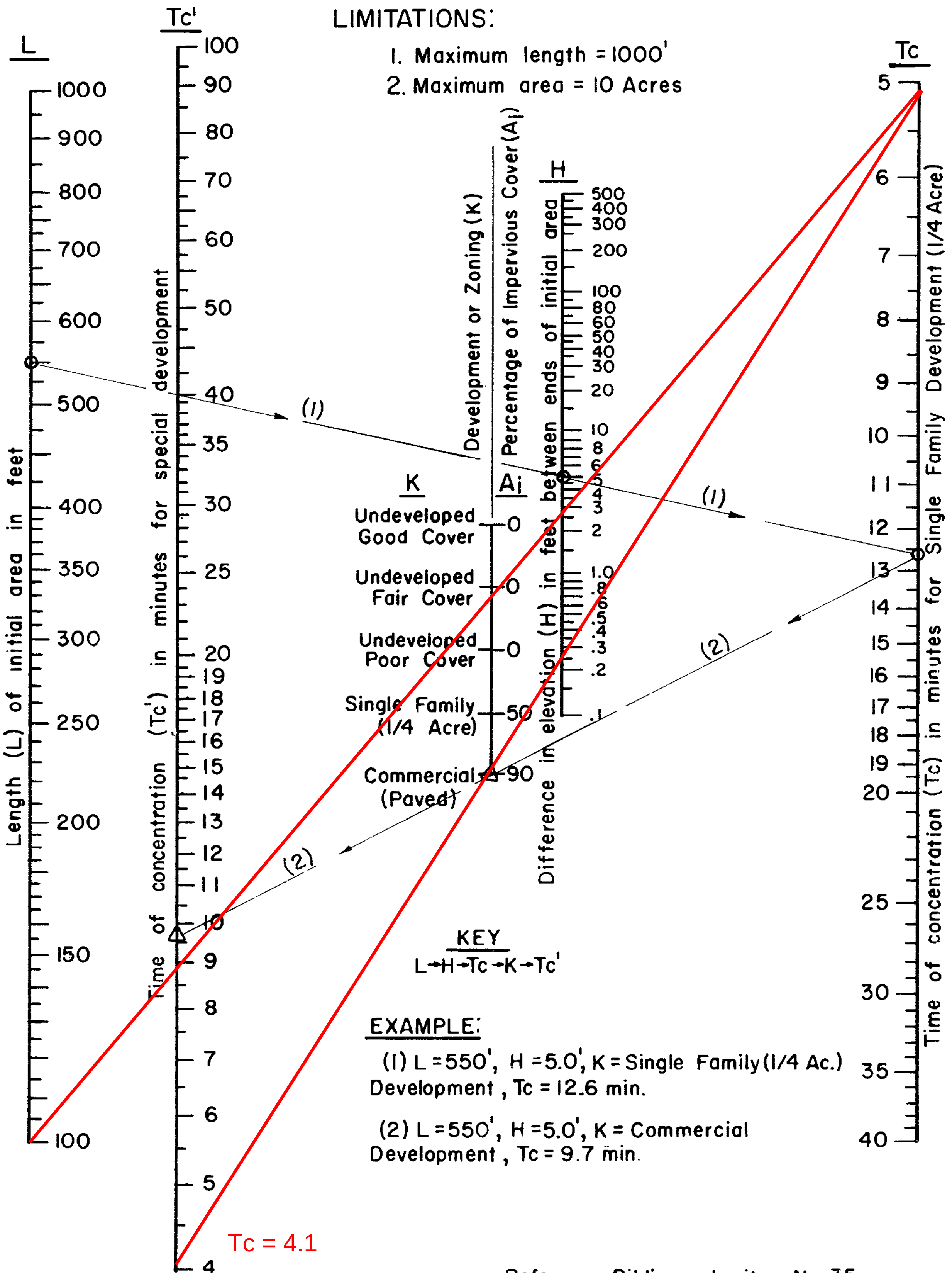
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 2



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

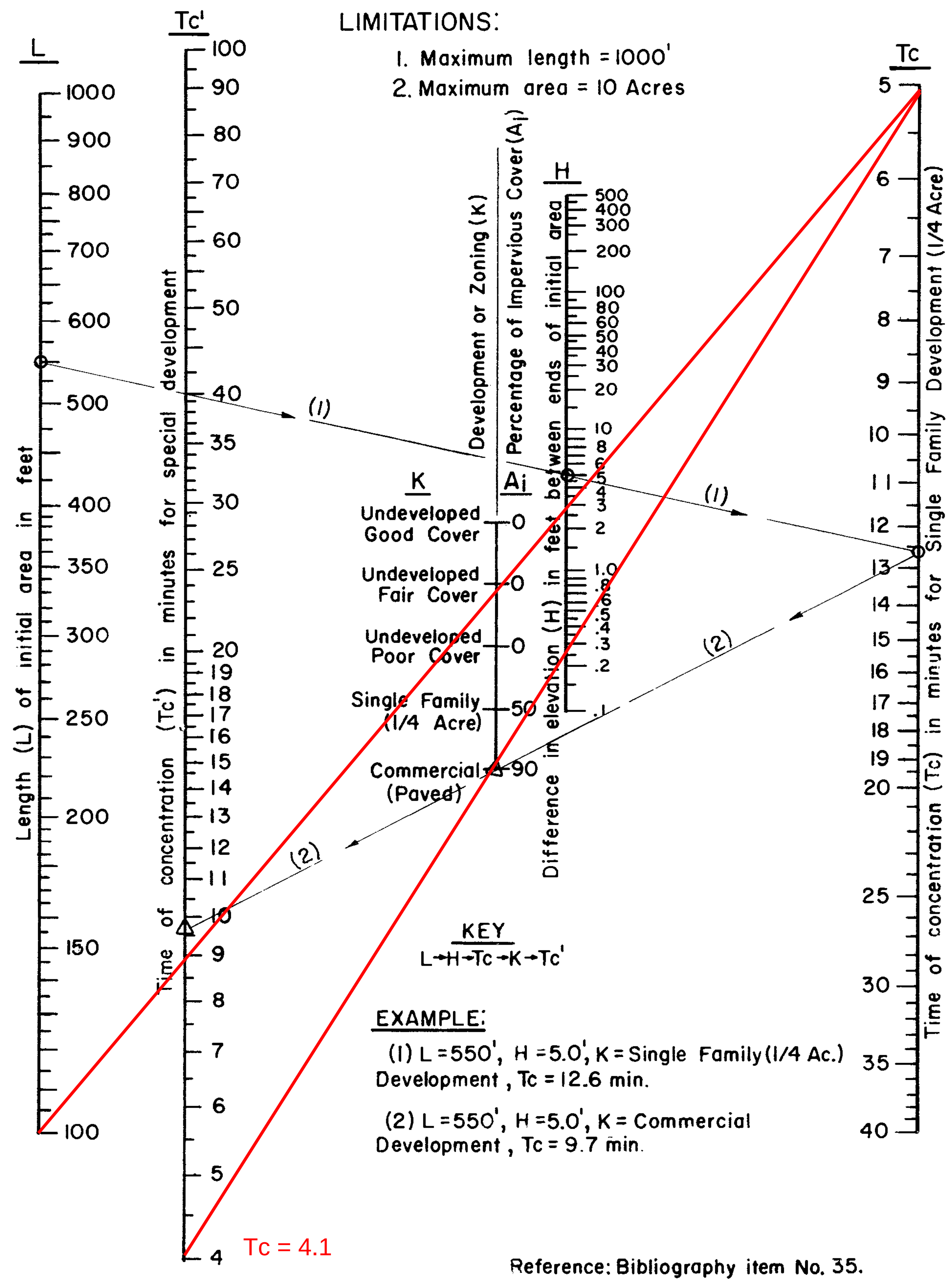
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 3



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

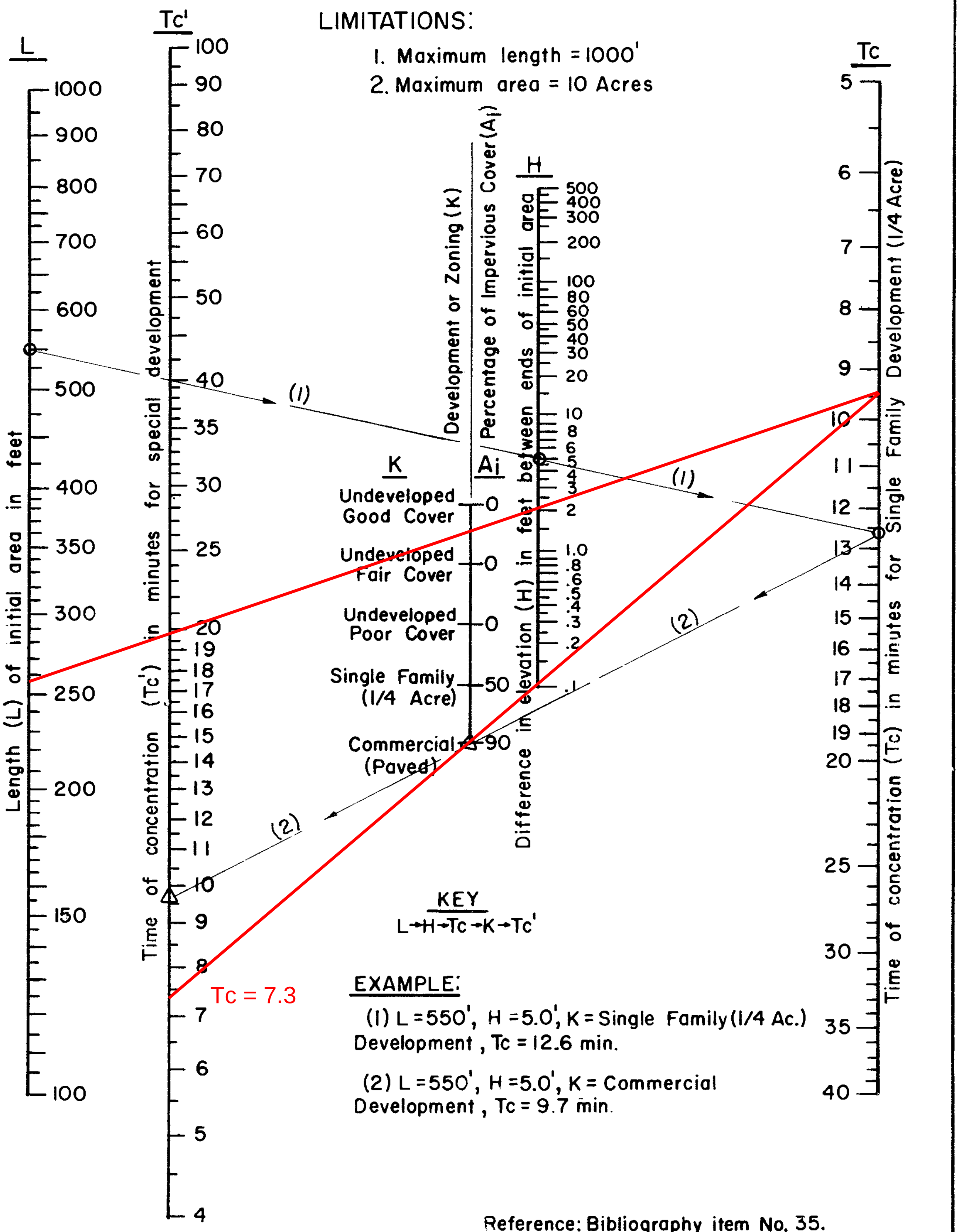
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 4



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

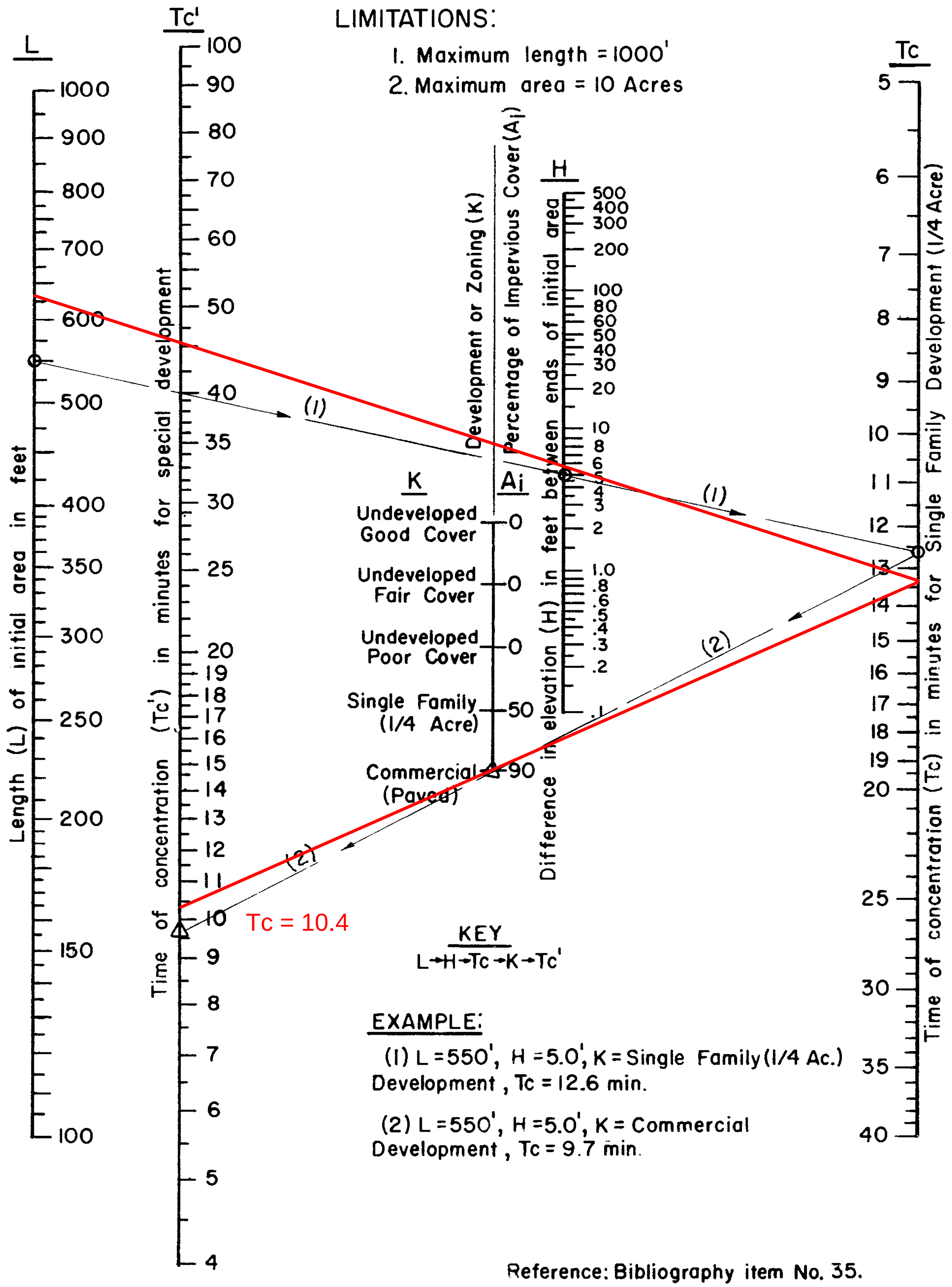
TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 5

LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres



KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

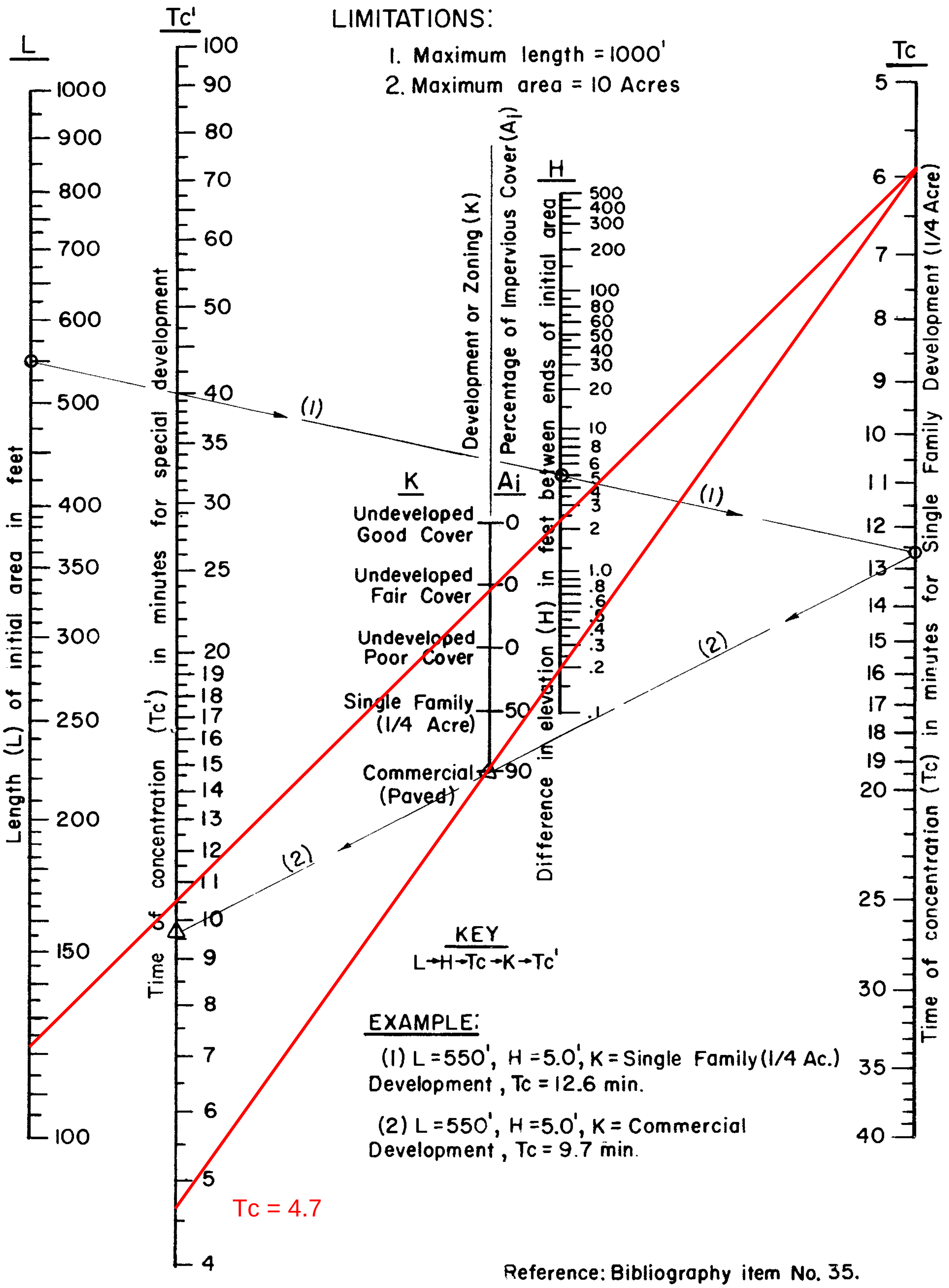
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 6



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

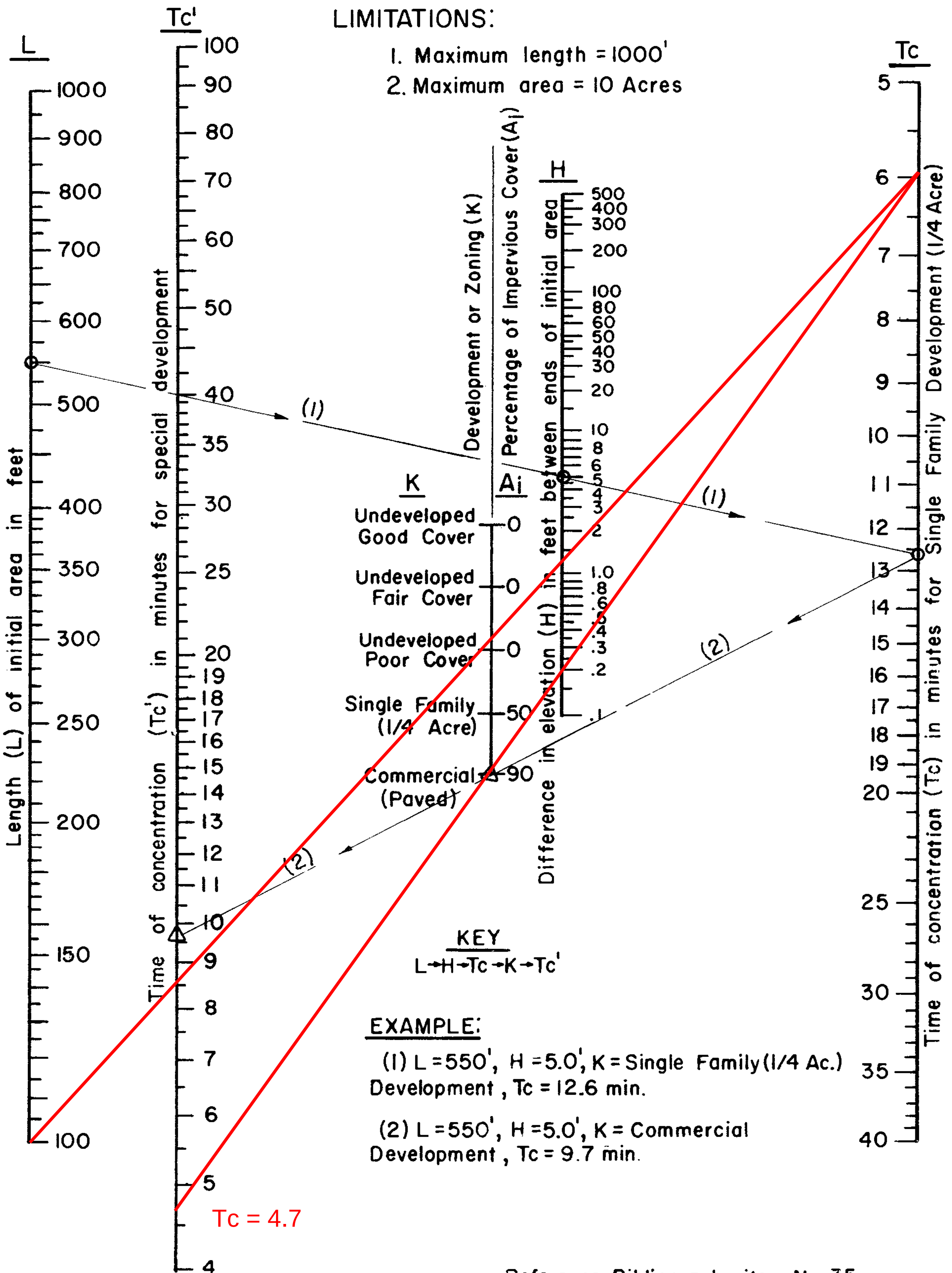
Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

AREA 7



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Tc = 4.7

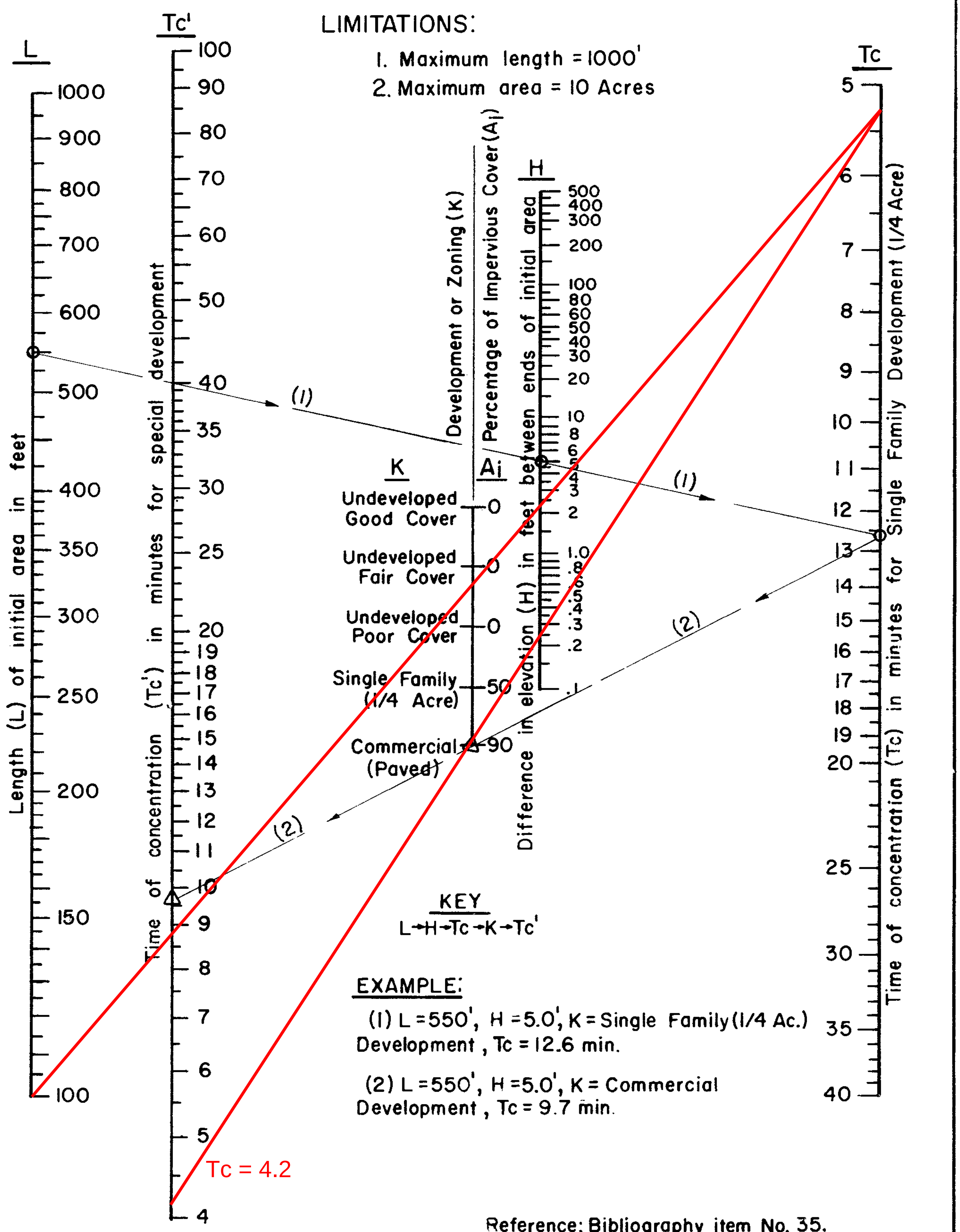
Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

AREA 8



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

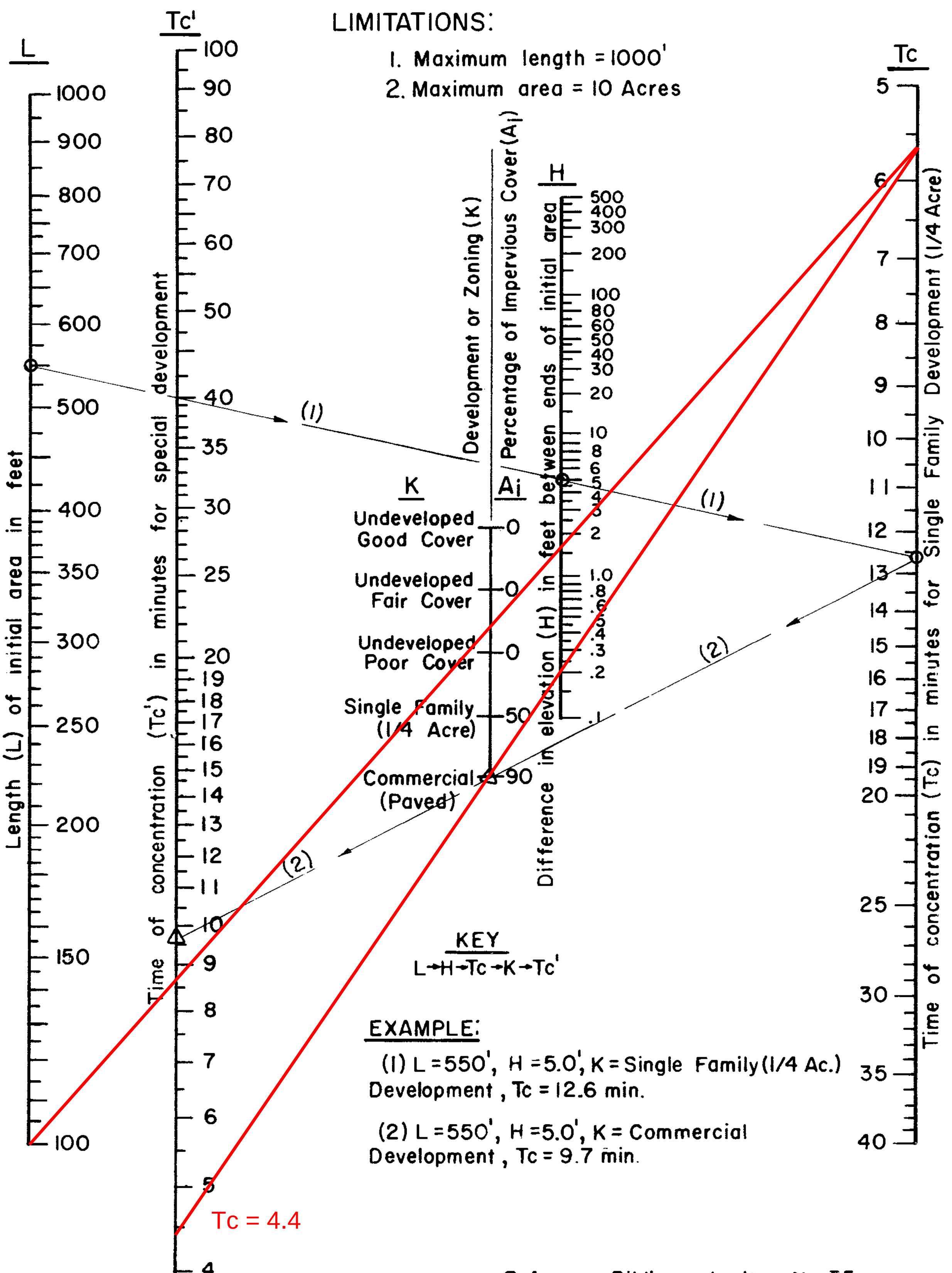
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 9



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

RCFC & WCD

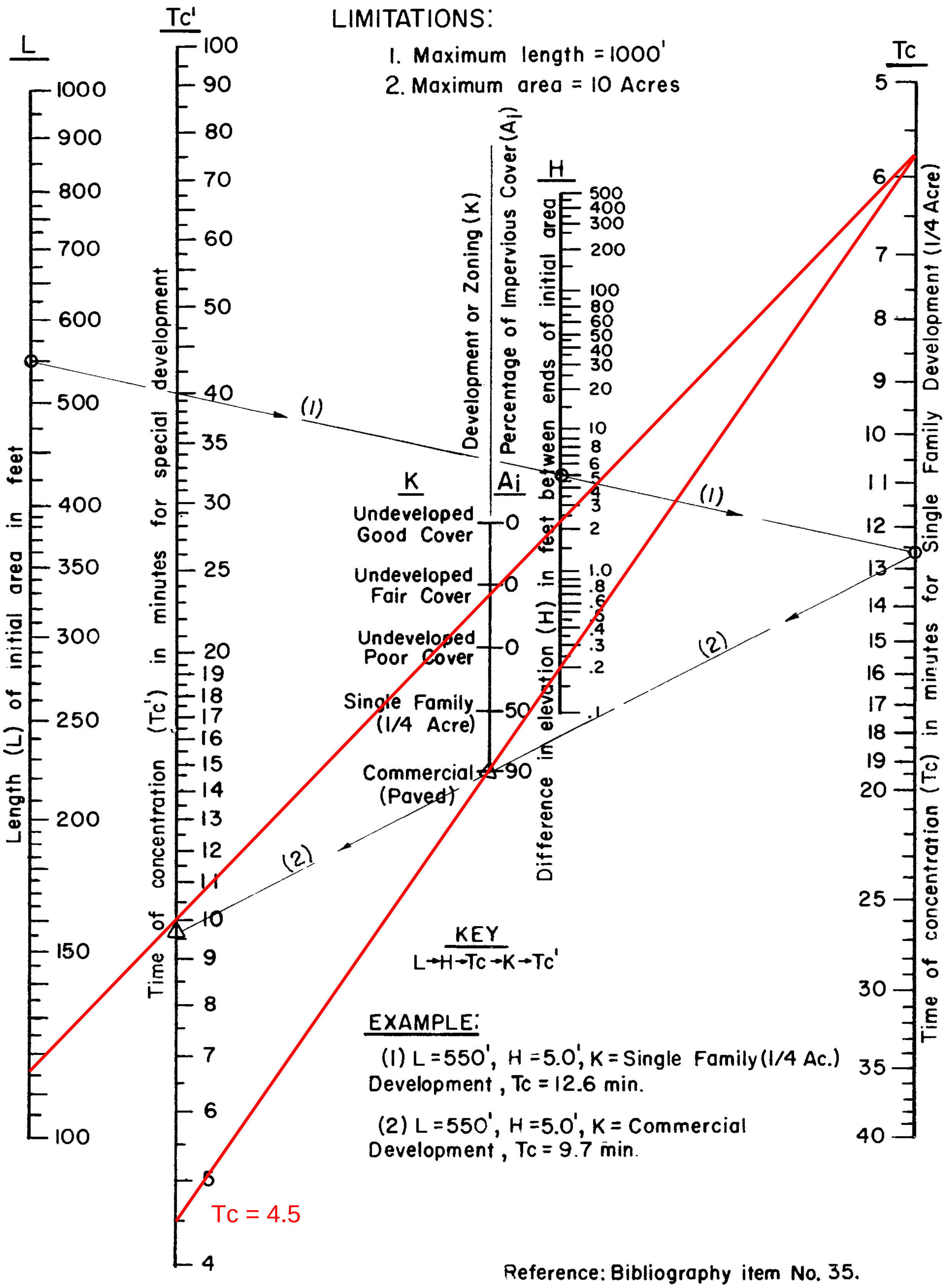
HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

AREA 10

LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres



KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

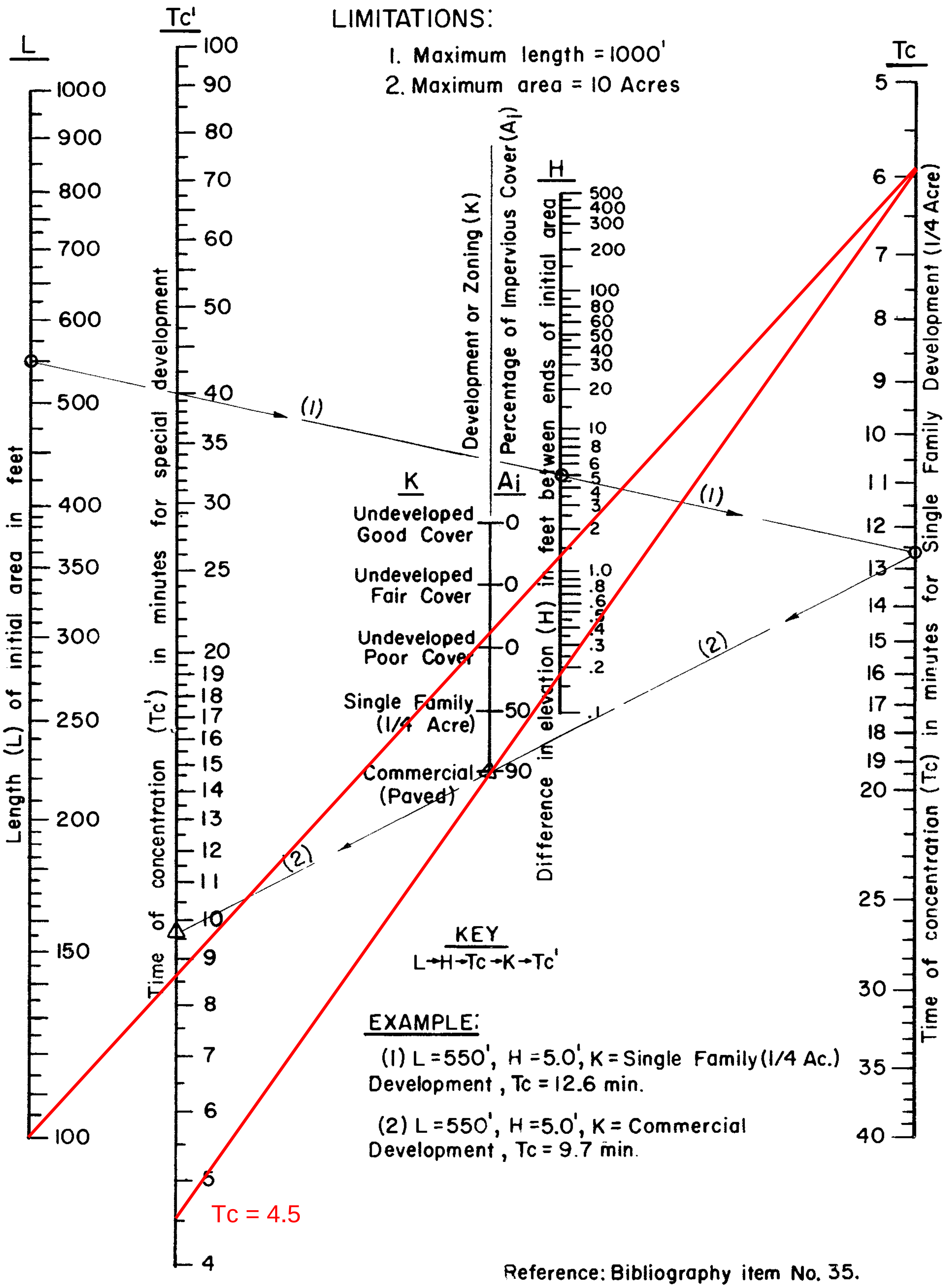
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 11



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

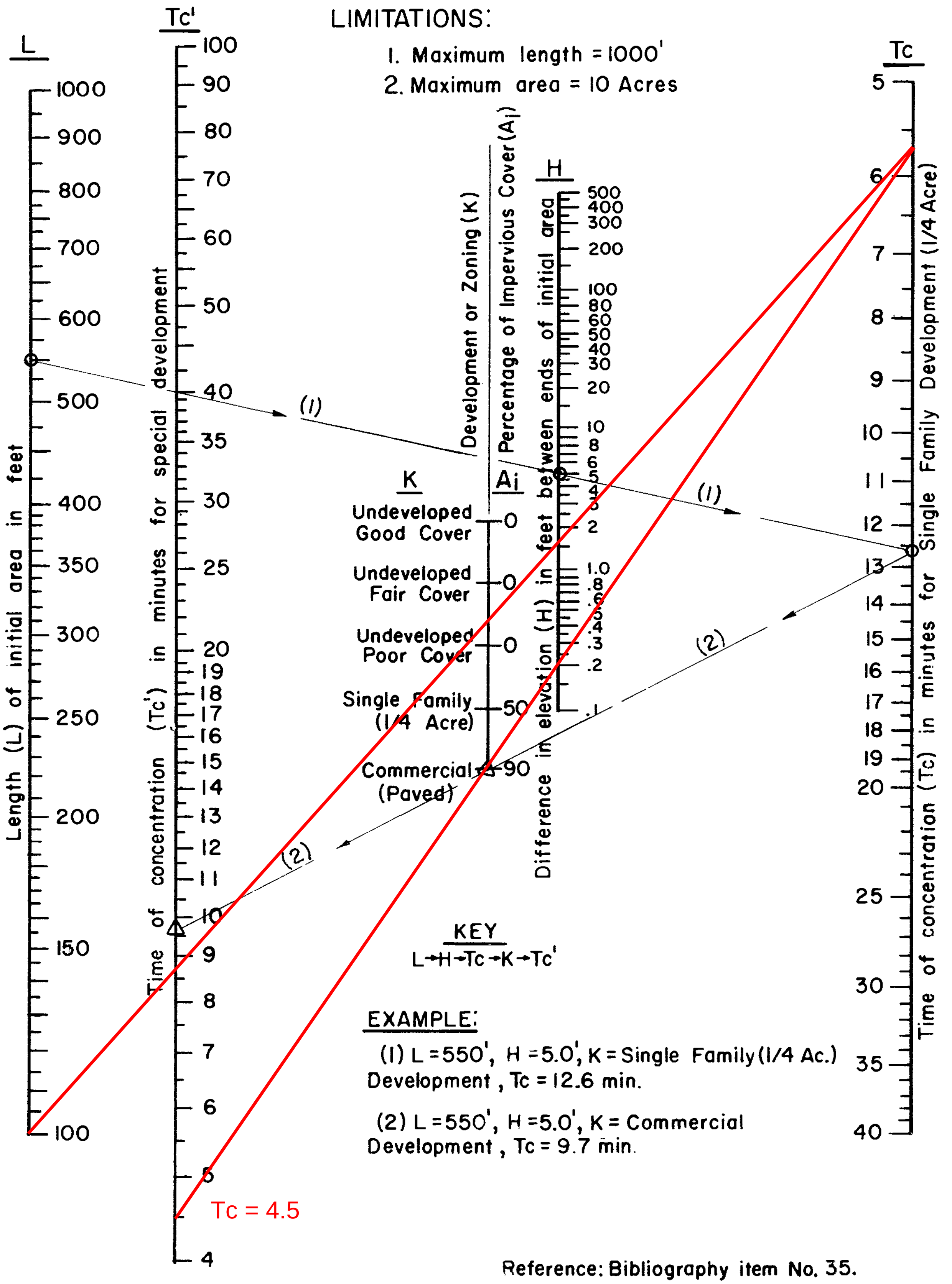
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 12



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

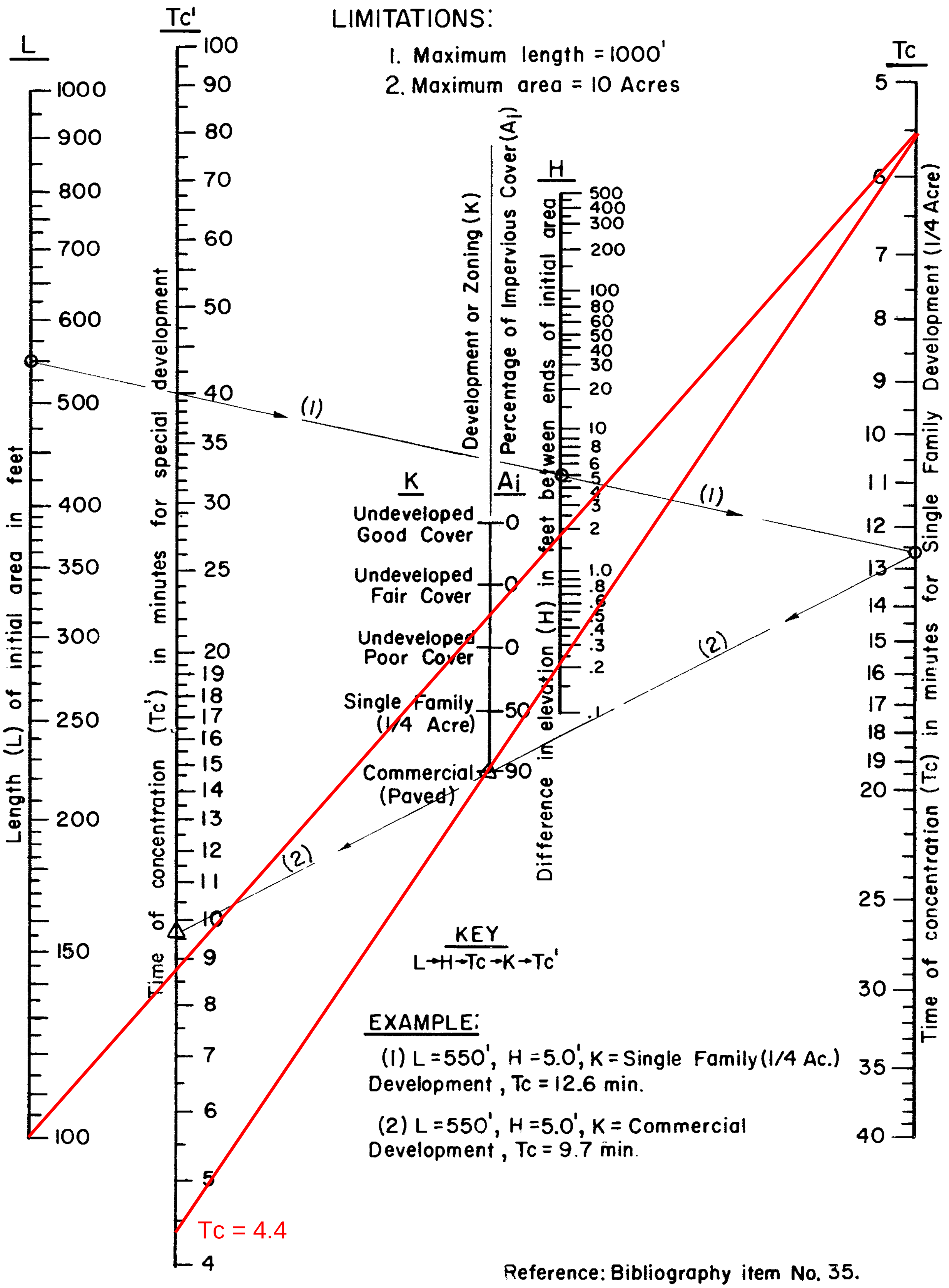
Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

AREA 13



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development , Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development , Tc = 9.7 min.

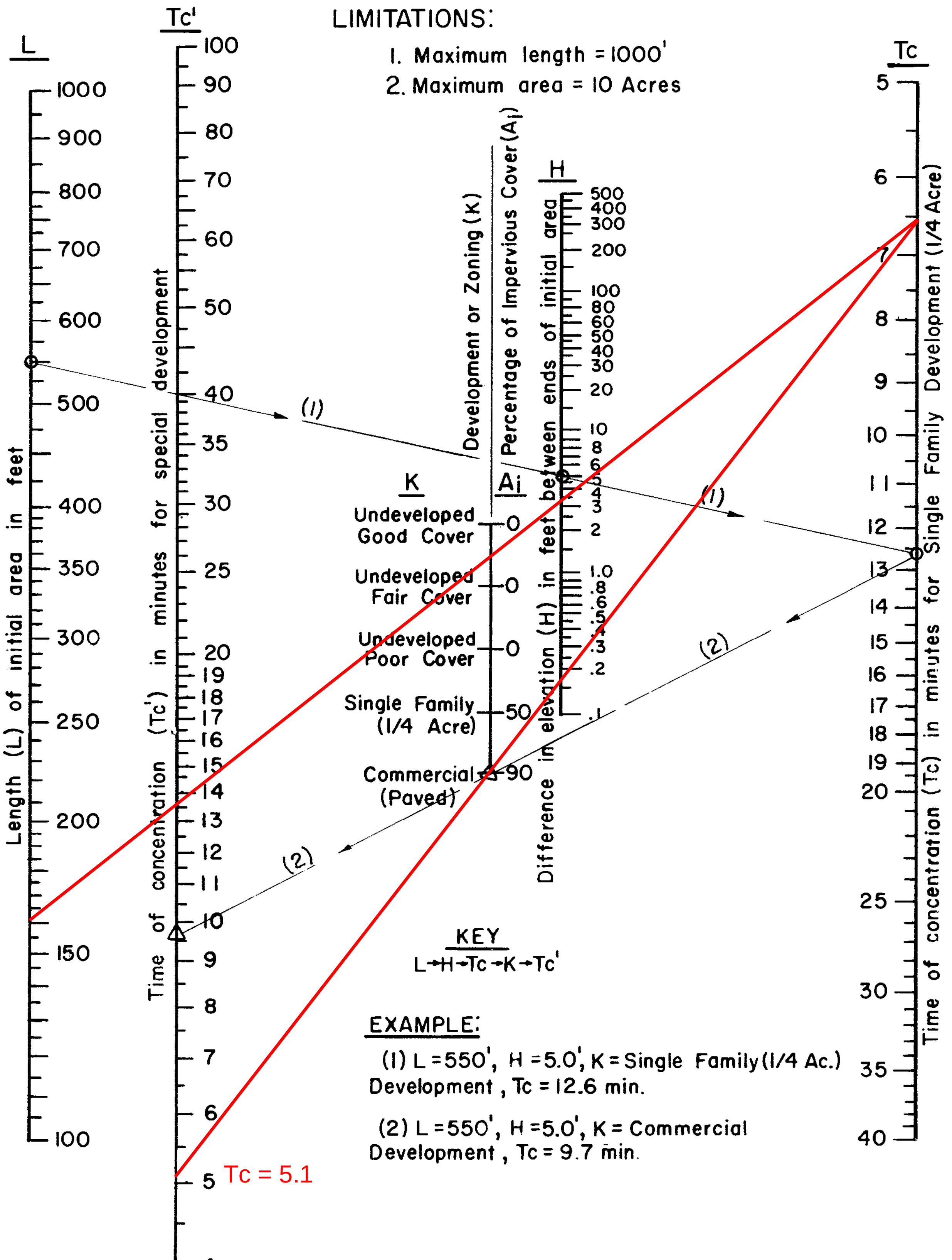
Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

AREA 14



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

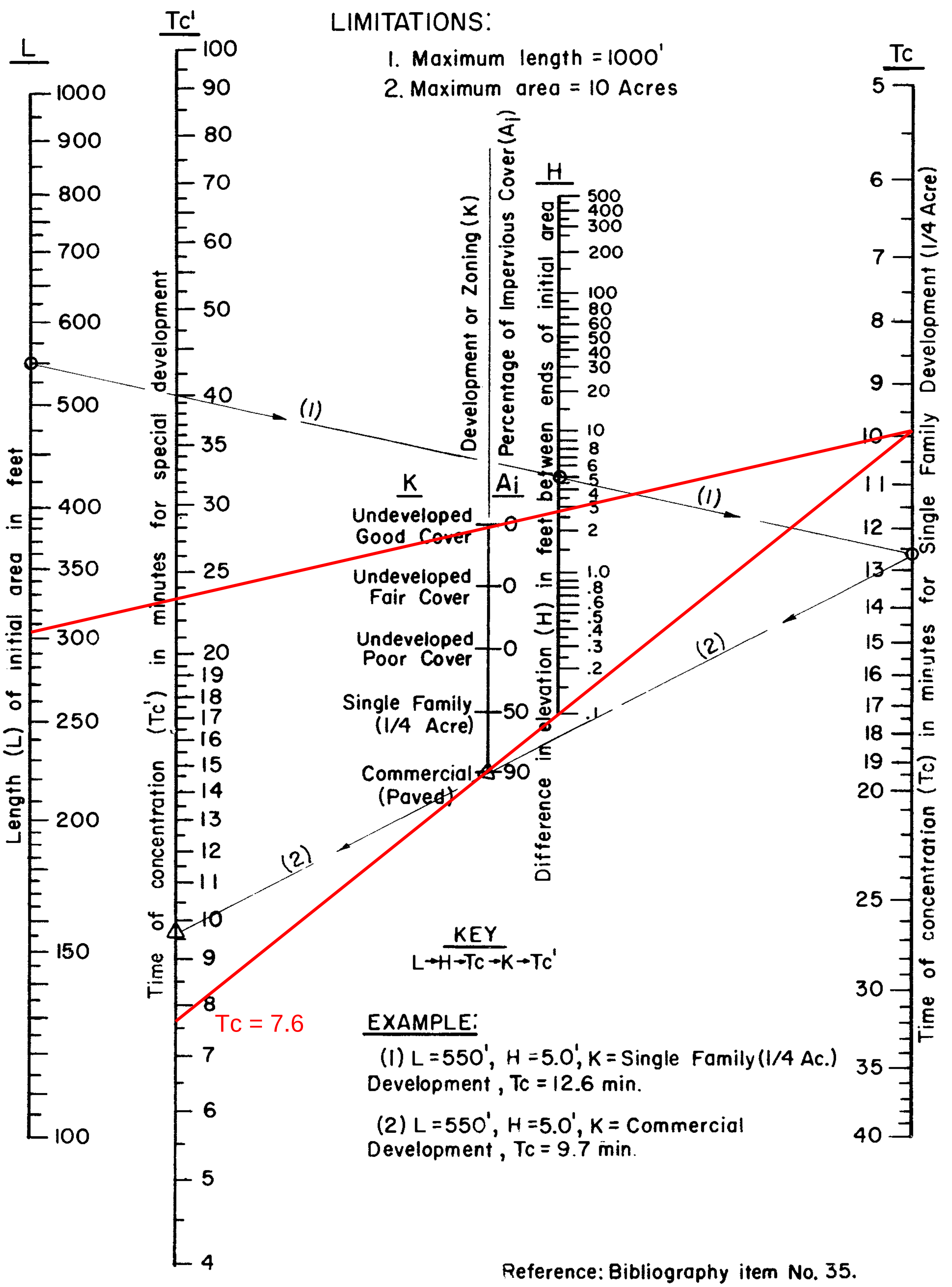
RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 15



LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres

KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

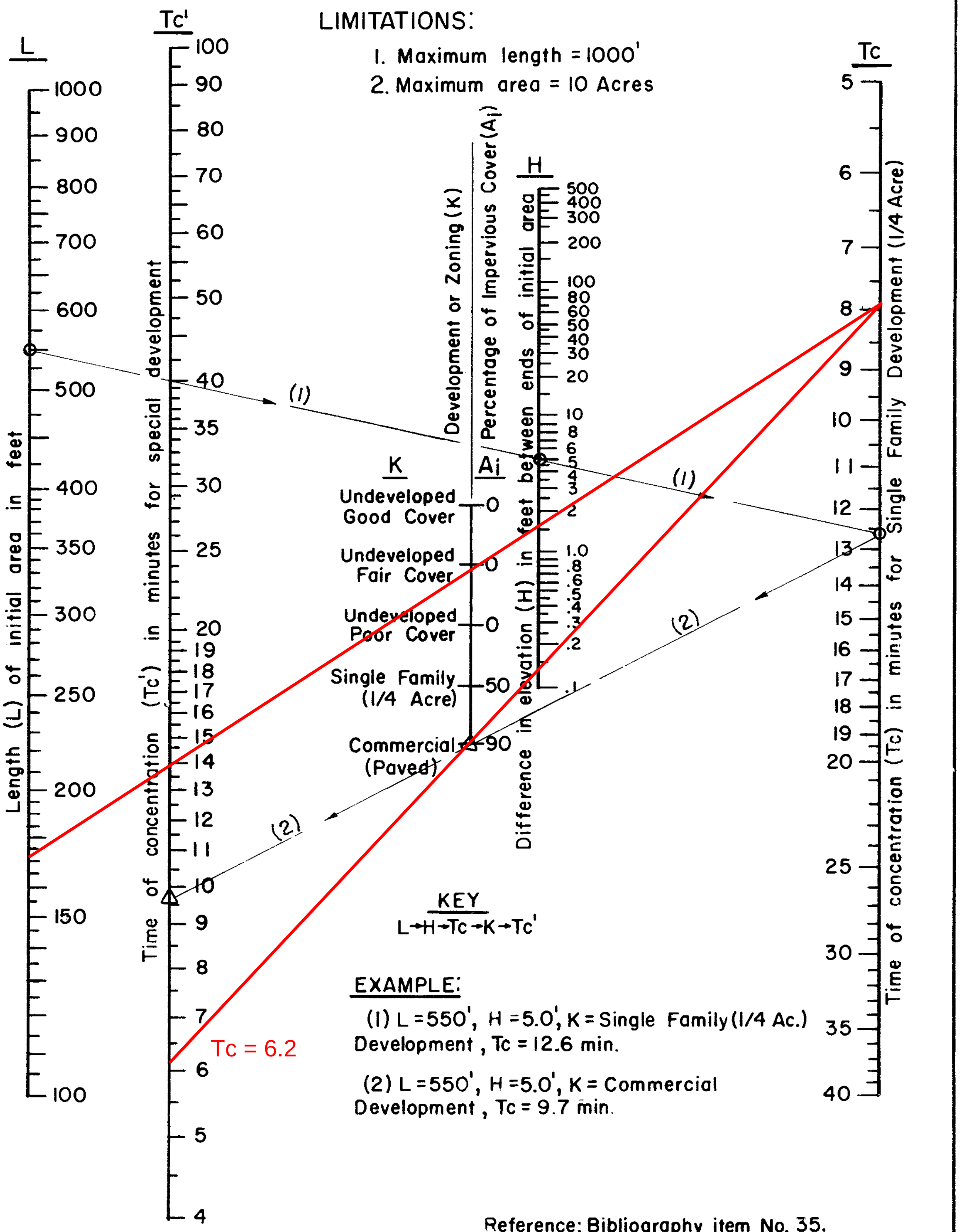
TIME OF CONCENTRATION FOR INITIAL SUBAREA

PLATE D-3

AREA 16

LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres



KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

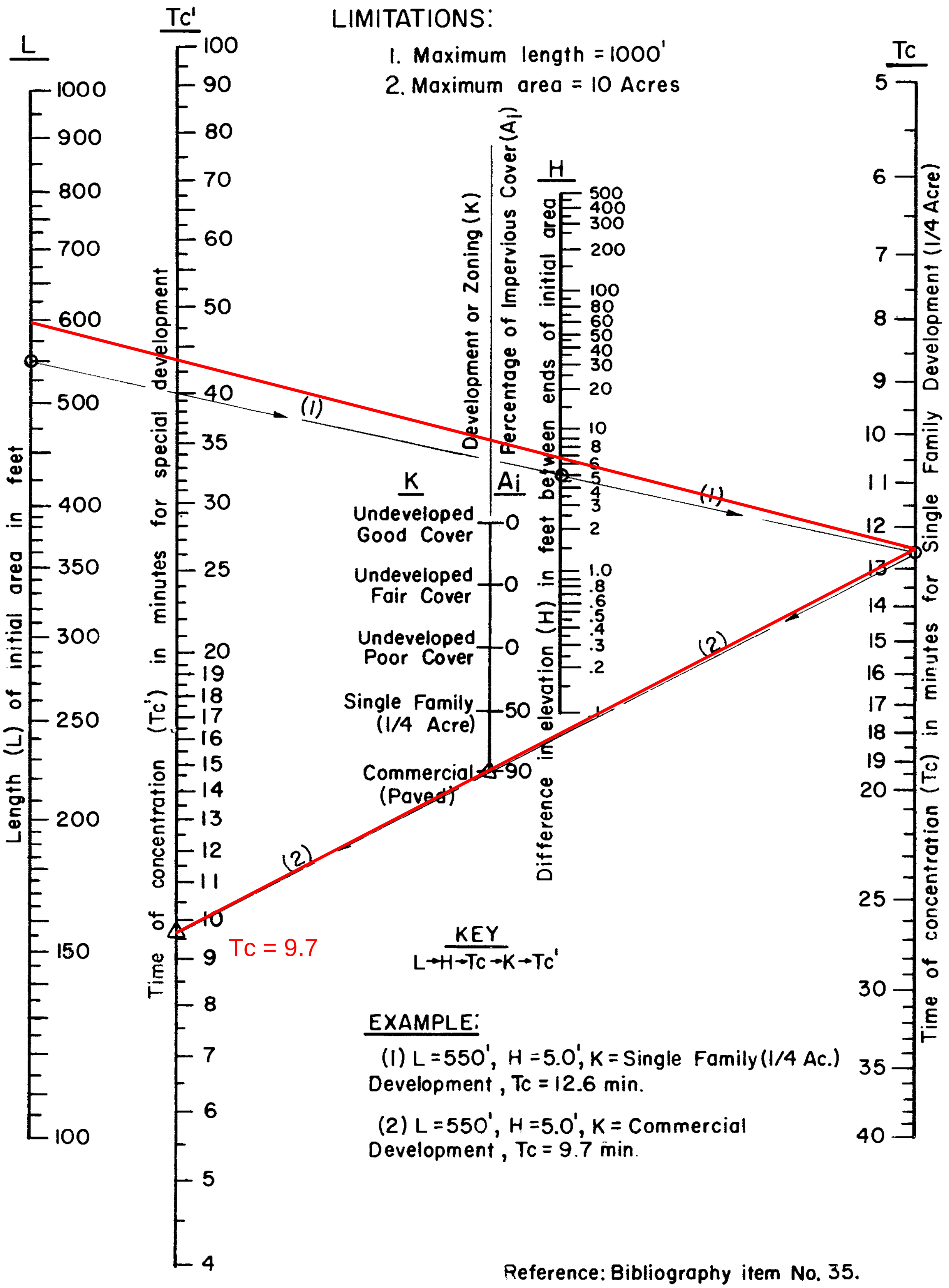
RCFC & WCD
HYDROLOGY MANUAL

TIME OF CONCENTRATION
FOR INITIAL SUBAREA

AREA 17

LIMITATIONS:

1. Maximum length = 1000'
2. Maximum area = 10 Acres



KEY

L→H→Tc→K→Tc'

EXAMPLE:

(1) L = 550', H = 5.0', K = Single Family (1/4 Ac.) Development, Tc = 12.6 min.

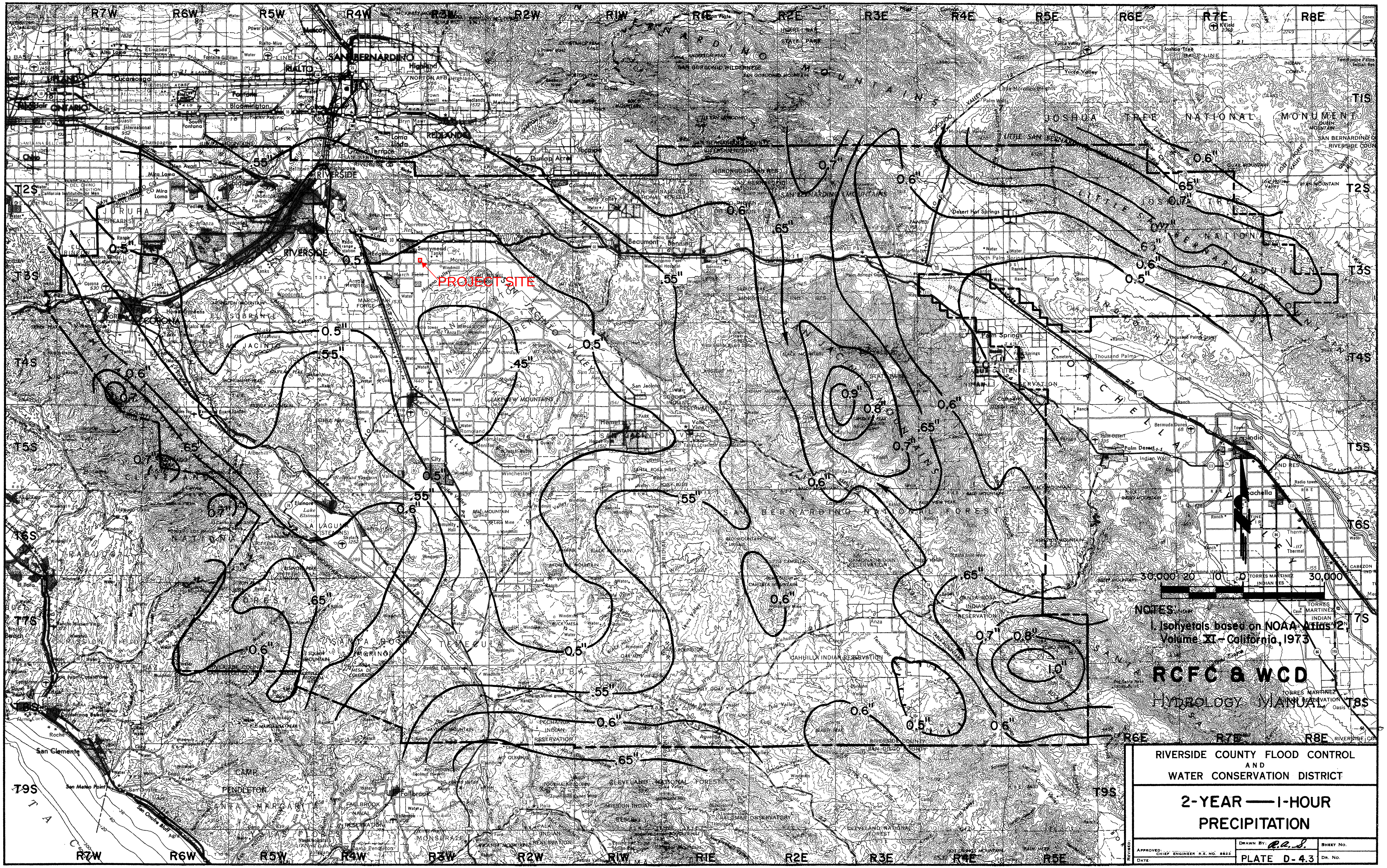
(2) L = 550', H = 5.0', K = Commercial Development, Tc = 9.7 min.

Reference: Bibliography item No. 35.

RCFC & WCD

HYDROLOGY MANUAL

TIME OF CONCENTRATION FOR INITIAL SUBAREA



NOTES:
1. Isohyetals based on NOAA Atlas 2, Volume XI - California, 1973

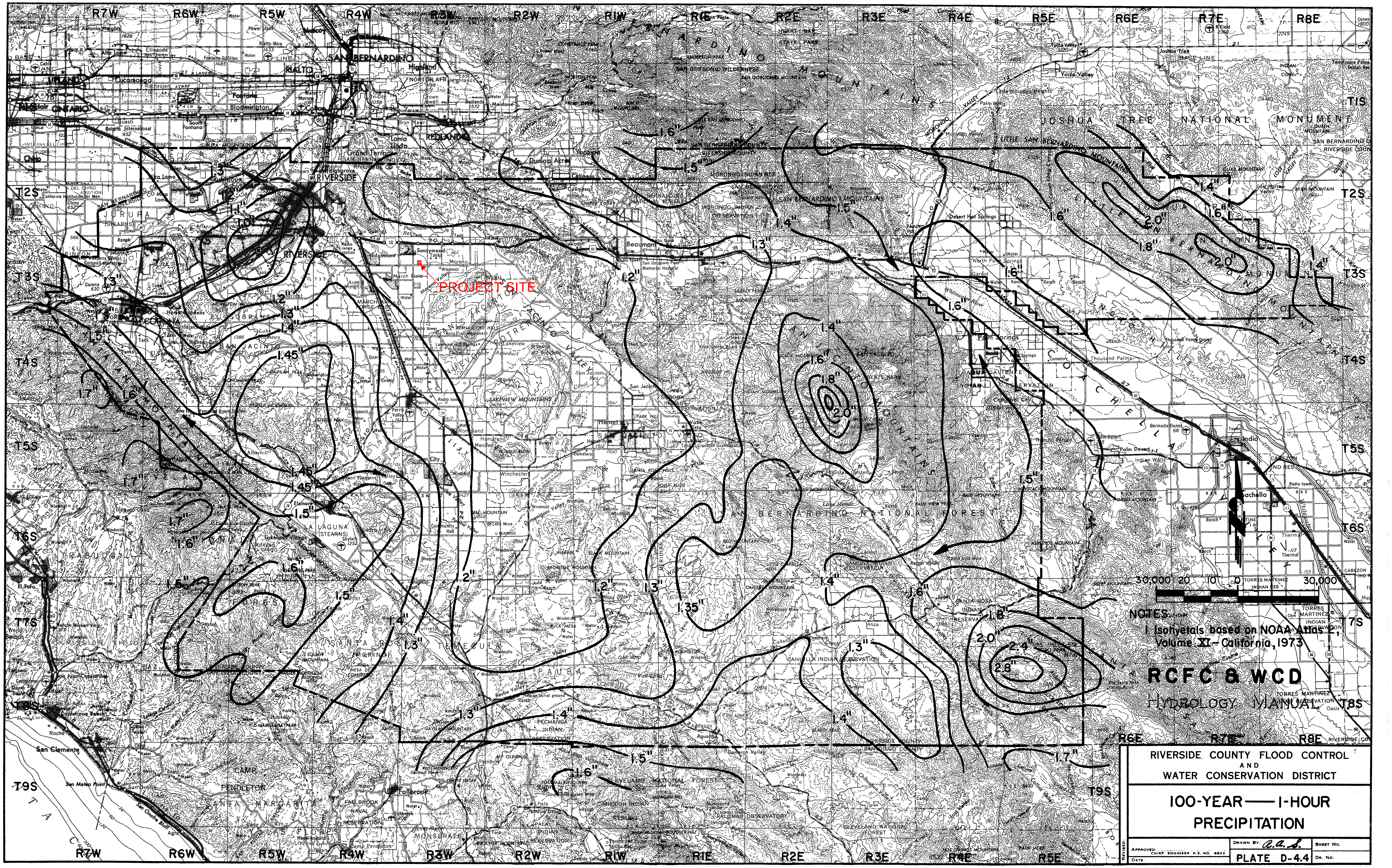
RCFC & WCD
HYDROLOGY MANUAL

RIVERSIDE COUNTY FLOOD CONTROL
AND
WATER CONSERVATION DISTRICT

2-YEAR —— 1-HOUR PRECIPITATION

APPROVED CHIEF ENGINEER R.E. NO. 8822	DRAWN BY: R.A.S.	SHEET NO.
DATE	PLATE D-4.3	DR. NO.

REVISED:

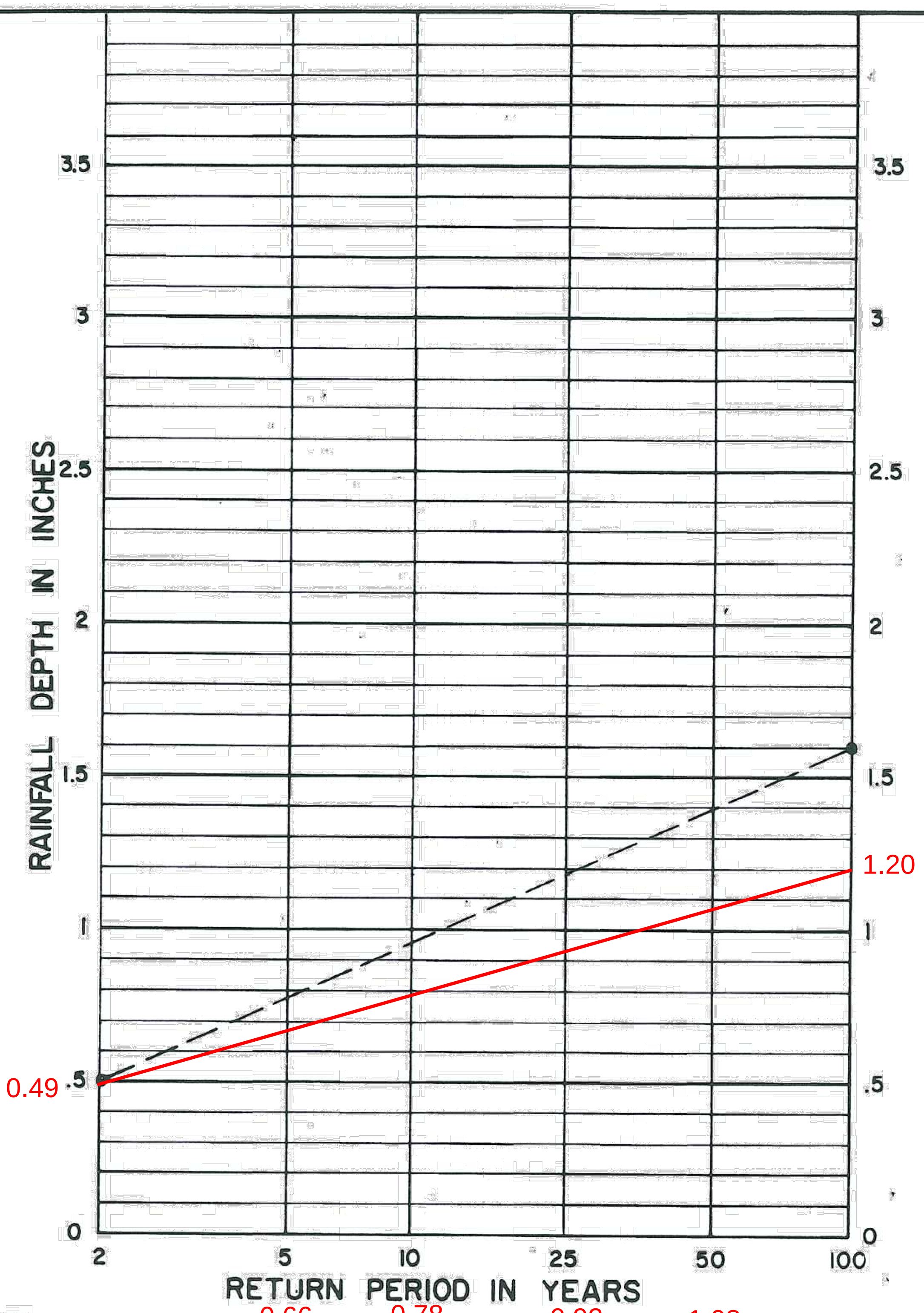


RIVERSIDE COUNTY FLOOD CONTROL
AND
WATER CONSERVATION DISTRICT

100-YEAR——1-HOUR PRECIPITATION

APPROVED: ________ CHIEF ENGINEER R.E. NO. 8822	DRAWN BY: R.A.S.	SHEET NO.
DATE:	PLATE D-4.4	DR. NO.

REVISED:



NOTE:

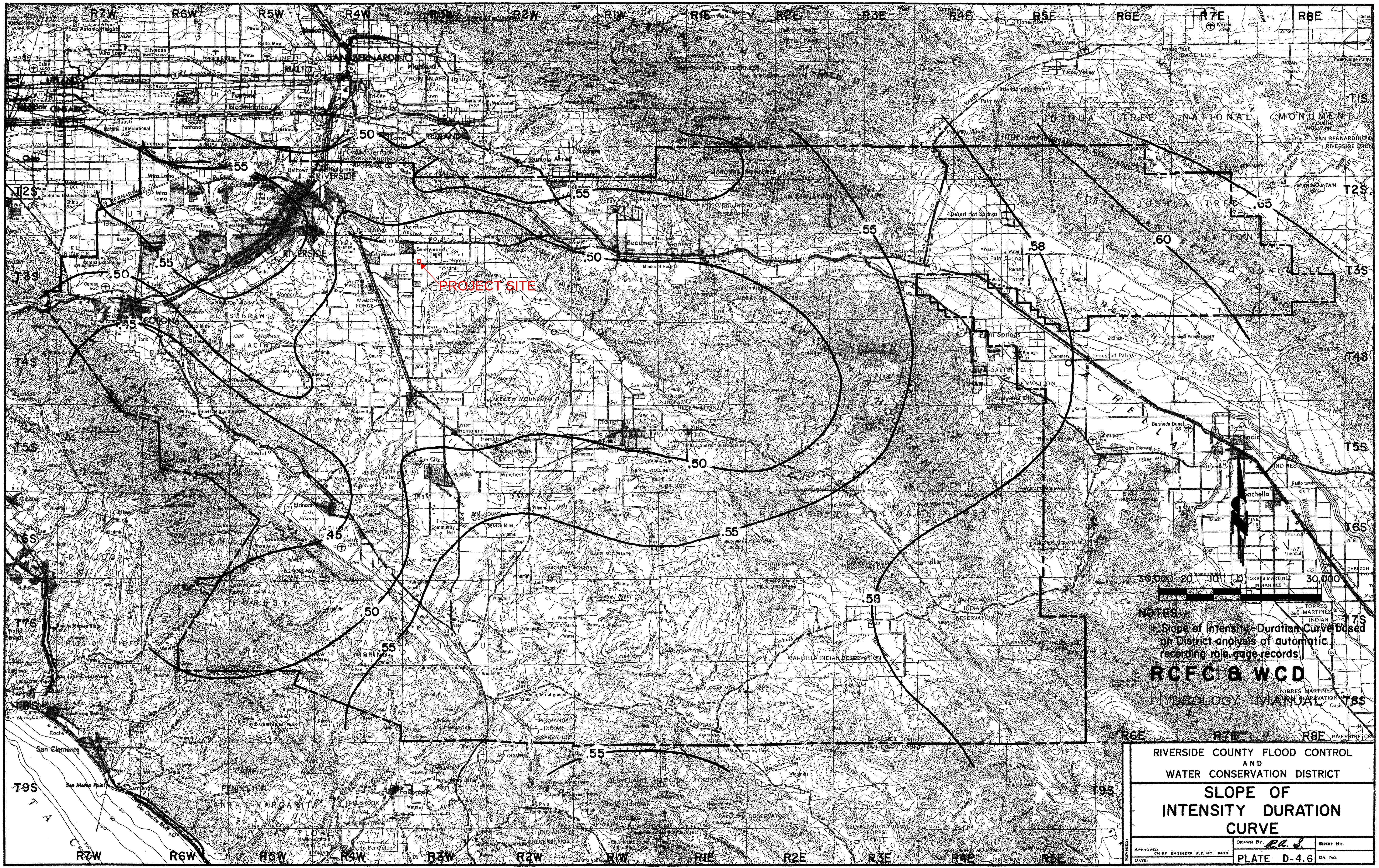
1. For intermediate return periods plot 2-year and 100-year one hour values from maps, then connect points and read value for desired return period. For example given 2-year one hour = .50" and 100-year one hour = 1.60", 25-year one hour = 1.18"

Reference: NOAA Atlas 2, Volume XI-California, 1973.

RCFC & WCD
HYDROLOGY MANUAL

RAINFALL DEPTH VERSUS RETURN PERIOD FOR PARTIAL DURATION SERIES

PLATE D-4.5



NOTES:

1. Slope of Intensity-Duration Curve based on District analysis of automatic recording rain gage records.

RCFC & WCD

HYDROLOGY MANUAL

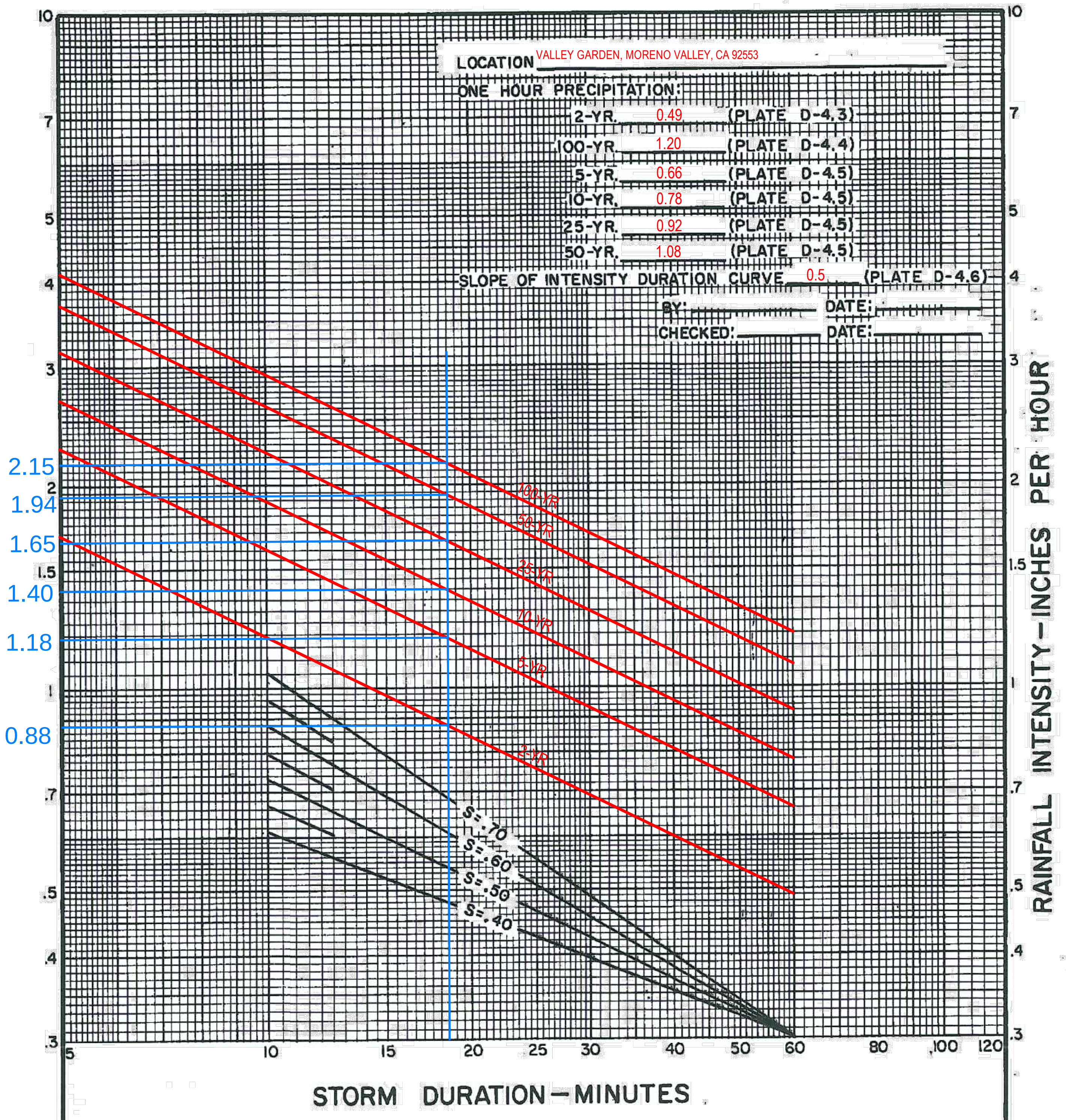
RIVERSIDE COUNTY FLOOD CONTROL
AND
WATER CONSERVATION DISTRICT

SLOPE OF INTENSITY DURATION CURVE

APPROVED: CHIEF ENGINEER R.E. NO. 8822	DRAWN BY: R.A.S.	SHEET NO.
DATE:	PLATE D-4.6	DR. NO.

REVISED:

EXISTING AREA



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)

100-YR. 1.20 (PLATE D-4.4)

5-YR. 0.66 (PLATE D-4.5)

10-YR. 0.78 (PLATE D-4.5)

25-YR. 0.92 (PLATE D-4.5)

50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

BY: ______ DATE: ______

CHECKED: ______ DATE: ______

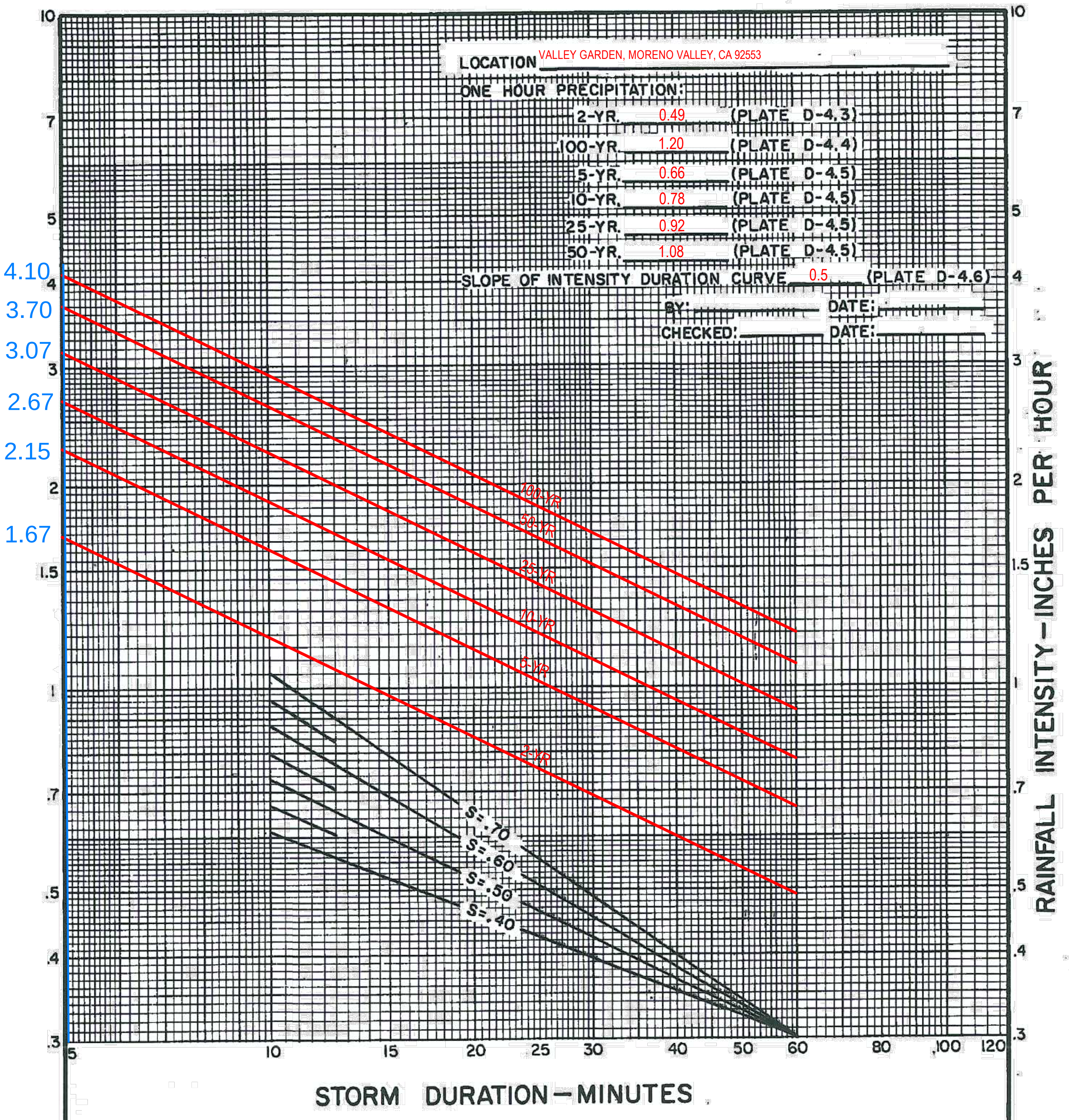
RCFC & WCD

HYDROLOGY MANUAL

INTENSITY—DURATION CURVES CALCULATION SHEET

PLATE D-4.7

AREA 1-3, 6-13



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)
100-YR. 1.20 (PLATE D-4.4)
5-YR. 0.66 (PLATE D-4.5)
10-YR. 0.78 (PLATE D-4.5)
25-YR. 0.92 (PLATE D-4.5)
50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

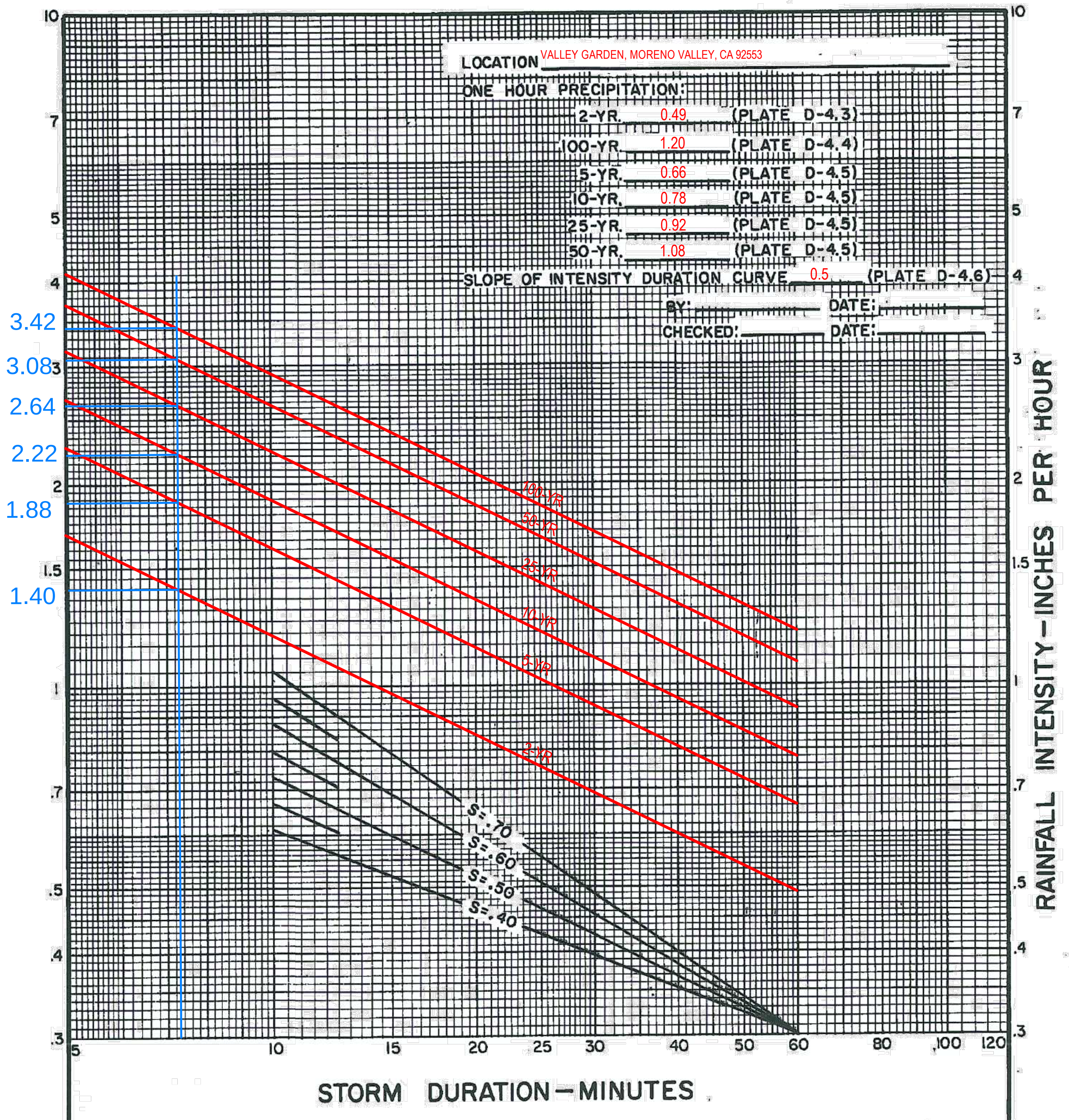
BY: ________ DATE: ________
CHECKED: ________ DATE: ________

RCFC & WCD
HYDROLOGY MANUAL

INTENSITY—DURATION CURVES CALCULATION SHEET

PLATE D-4.7

AREA 4



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)

100-YR. 1.20 (PLATE D-4.4)

5-YR. 0.66 (PLATE D-4.5)

10-YR. 0.78 (PLATE D-4.5)

25-YR. 0.92 (PLATE D-4.5)

50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

BY: ______ DATE: ______

CHECKED: ______ DATE: ______

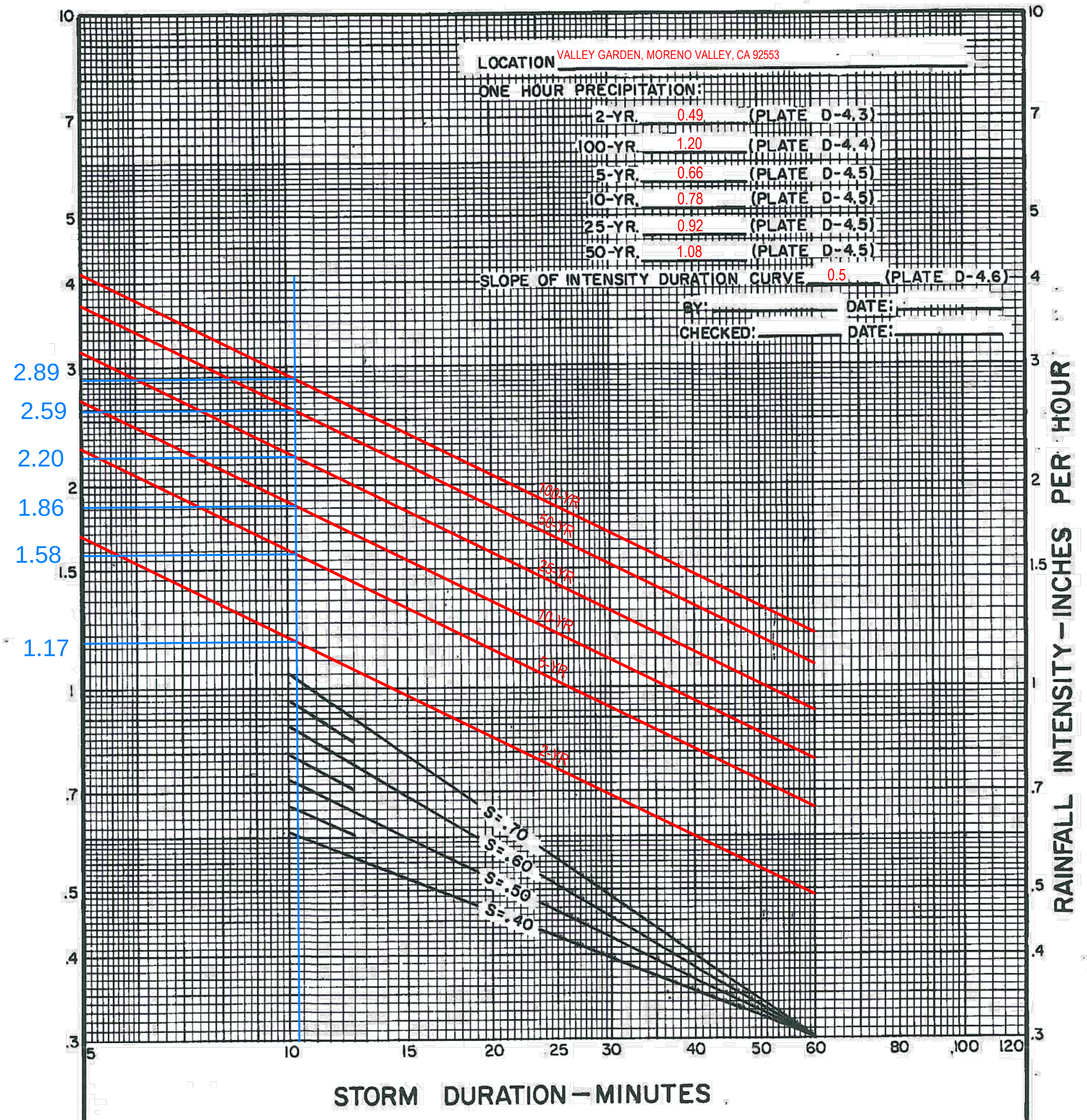
RCFC & WCD

HYDROLOGY MANUAL

INTENSITY–DURATION CURVES CALCULATION SHEET

PLATE D-4.7

AREA 5



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)

100-YR. 1.20 (PLATE D-4.4)

5-YR. 0.66 (PLATE D-4.5)

10-YR. 0.78 (PLATE D-4.5)

25-YR. 0.92 (PLATE D-4.5)

50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

BY: DATE:

CHECKED: DATE:

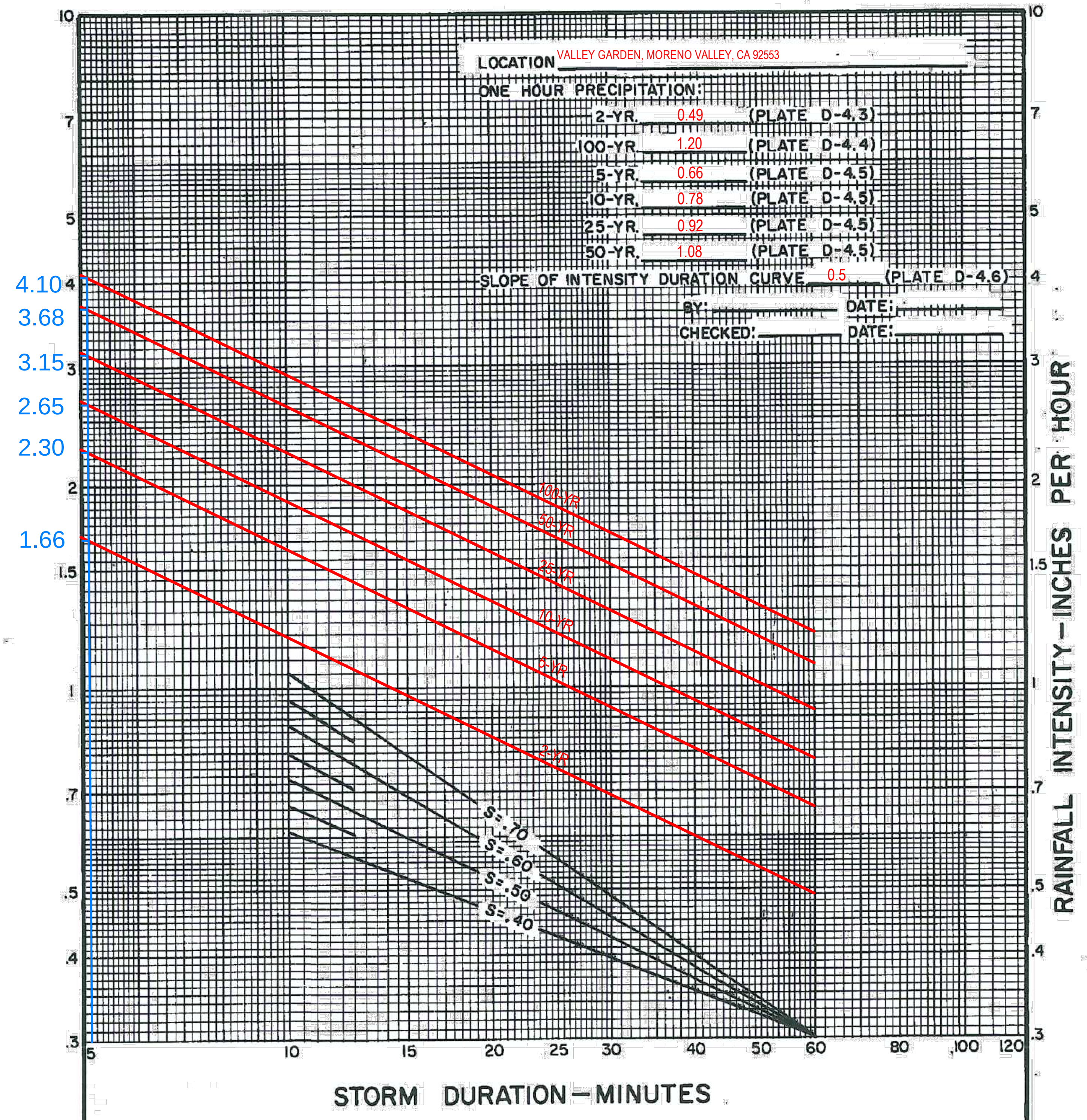
RCFC & WCD

HYDROLOGY MANUAL

INTENSITY—DURATION CURVES CALCULATION SHEET

PLATE D-4.7

AREA 14



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)

100-YR. 1.20 (PLATE D-4.4)

5-YR. 0.66 (PLATE D-4.5)

10-YR. 0.78 (PLATE D-4.5)

25-YR. 0.92 (PLATE D-4.5)

50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

BY: DATE:

CHECKED: DATE:

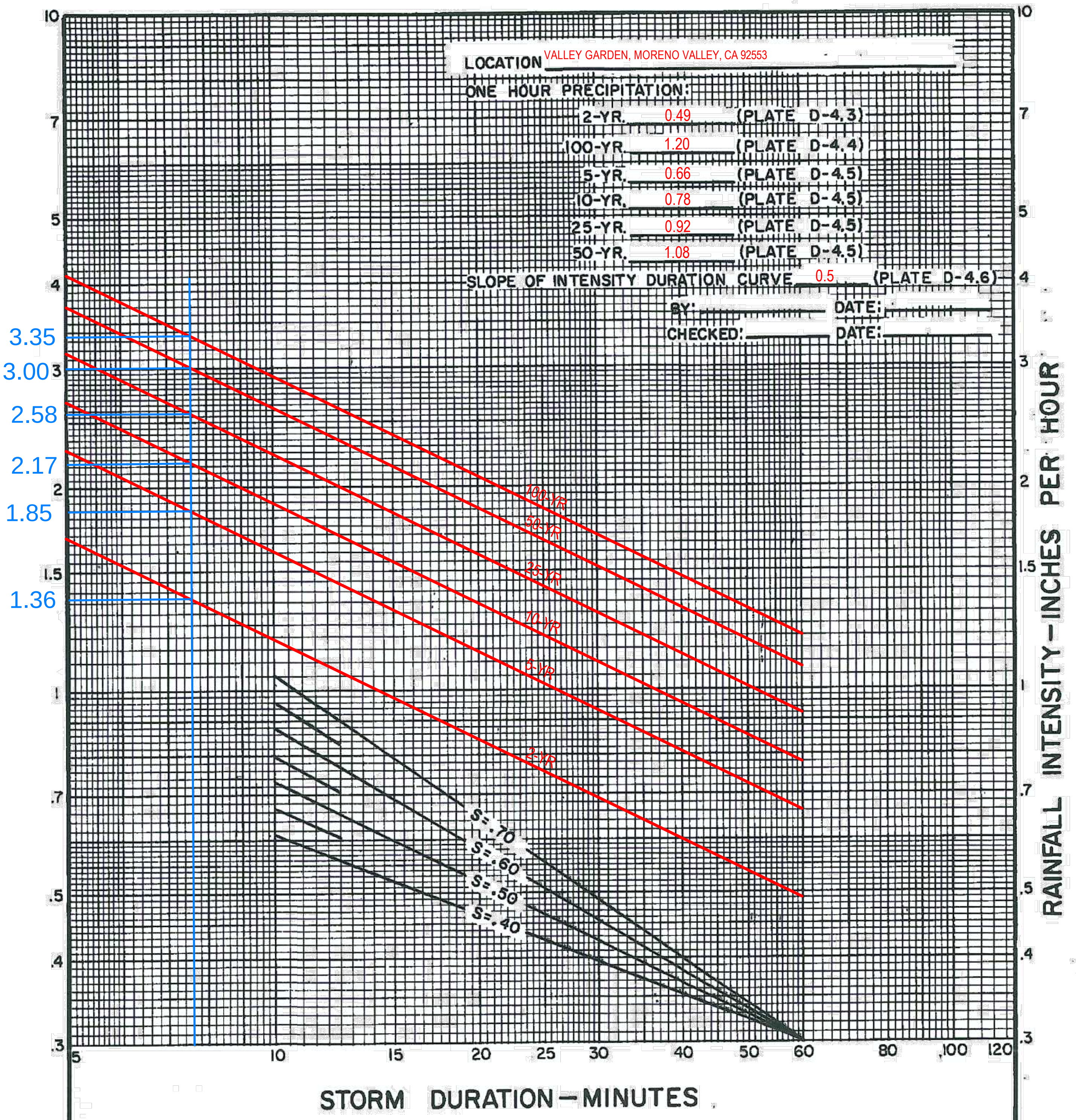
RCFC & WCD

HYDROLOGY MANUAL

INTENSITY—DURATION CURVES CALCULATION SHEET

PLATE D-4.7

AREA 15



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)

100-YR. 1.20 (PLATE D-4.4)

5-YR. 0.66 (PLATE D-4.5)

10-YR. 0.78 (PLATE D-4.5)

25-YR. 0.92 (PLATE D-4.5)

50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

BY: ________ DATE: ________

CHECKED: ________ DATE: ________

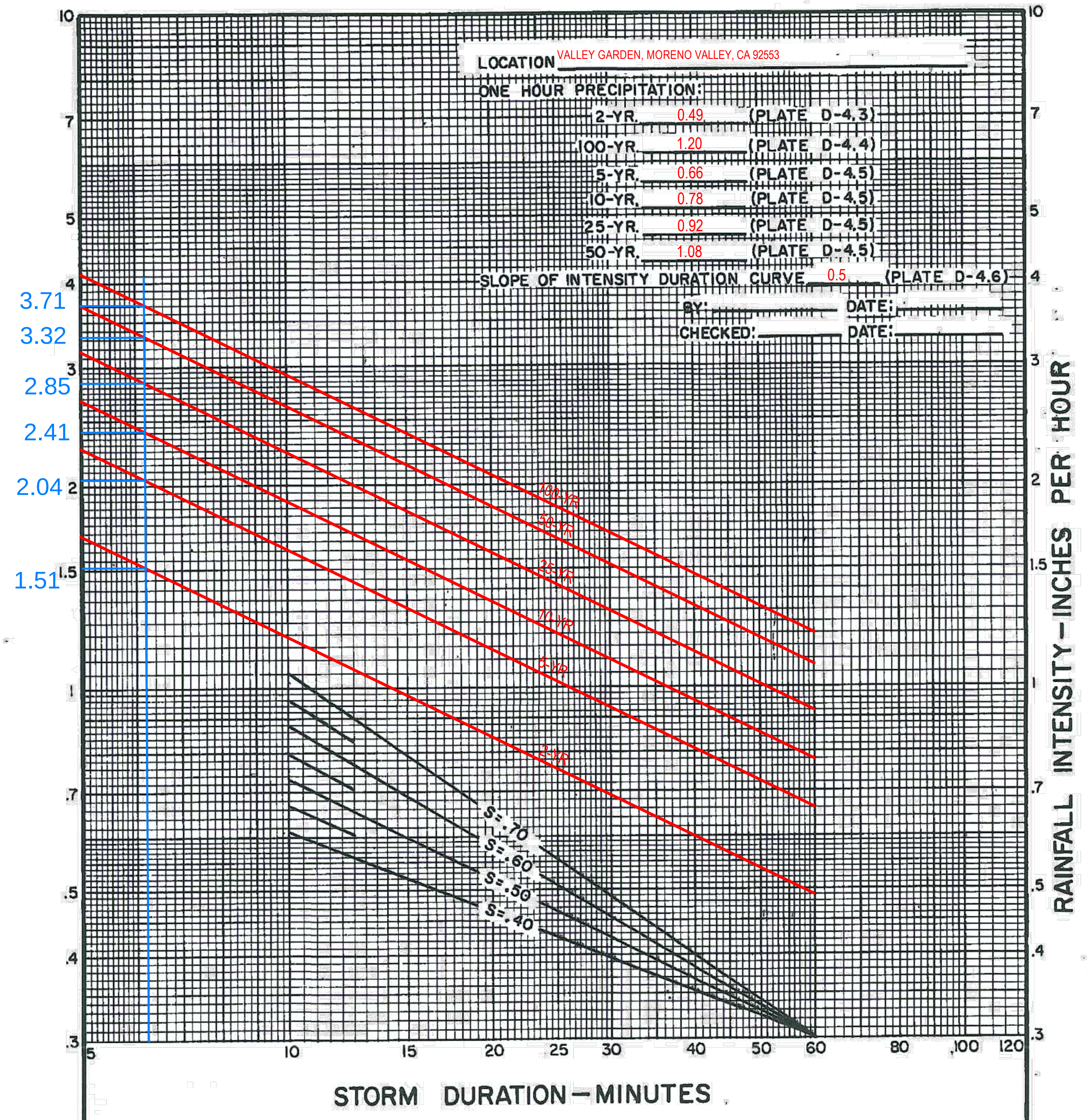
RCFC & WCD

HYDROLOGY MANUAL

INTENSITY—DURATION
CURVES
CALCULATION SHEET

PLATE D-4.7

AREA 16



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)
100-YR. 1.20 (PLATE D-4.4)
5-YR. 0.66 (PLATE D-4.5)
10-YR. 0.78 (PLATE D-4.5)
25-YR. 0.92 (PLATE D-4.5)
50-YR. 1.08 (PLATE D-4.5)

SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

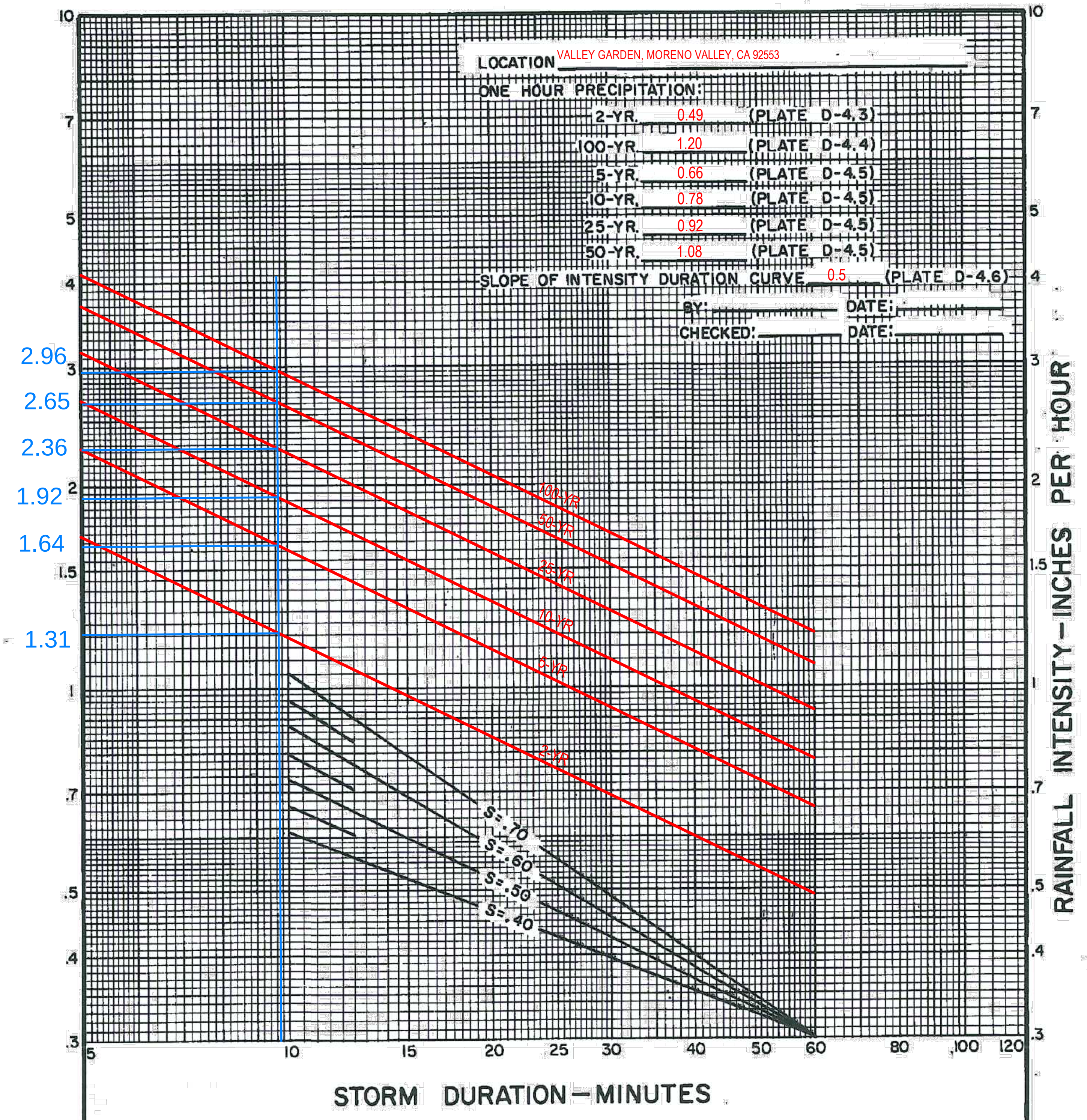
BY: ________ DATE: ________
CHECKED: ________ DATE: ________

RCFC & WCD
HYDROLOGY MANUAL

INTENSITY—DURATION CURVES CALCULATION SHEET

PLATE D-4.7

AREA 17



LOCATION VALLEY GARDEN, MORENO VALLEY, CA 92553

ONE HOUR PRECIPITATION:

2-YR. 0.49 (PLATE D-4.3)
100-YR. 1.20 (PLATE D-4.4)
5-YR. 0.66 (PLATE D-4.5)
10-YR. 0.78 (PLATE D-4.5)
25-YR. 0.92 (PLATE D-4.5)
50-YR. 1.08 (PLATE D-4.5)

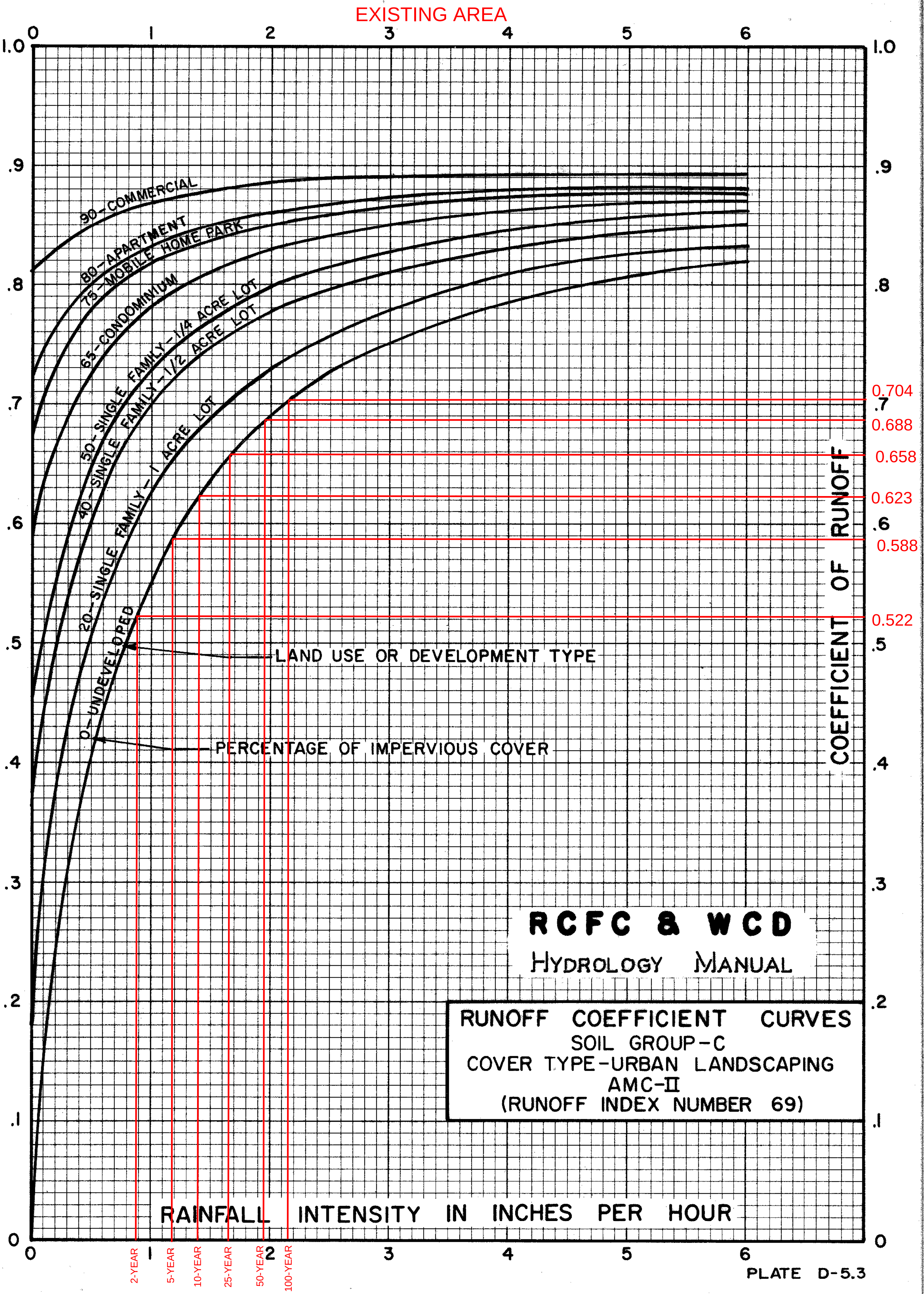
SLOPE OF INTENSITY DURATION CURVE 0.5 (PLATE D-4.6)

BY: DATE:
CHECKED: DATE:

RCFC & WCD
HYDROLOGY MANUAL

INTENSITY-DURATION CURVES CALCULATION SHEET

PLATE D-4.7



RCFC & WCD

HYDROLOGY MANUAL

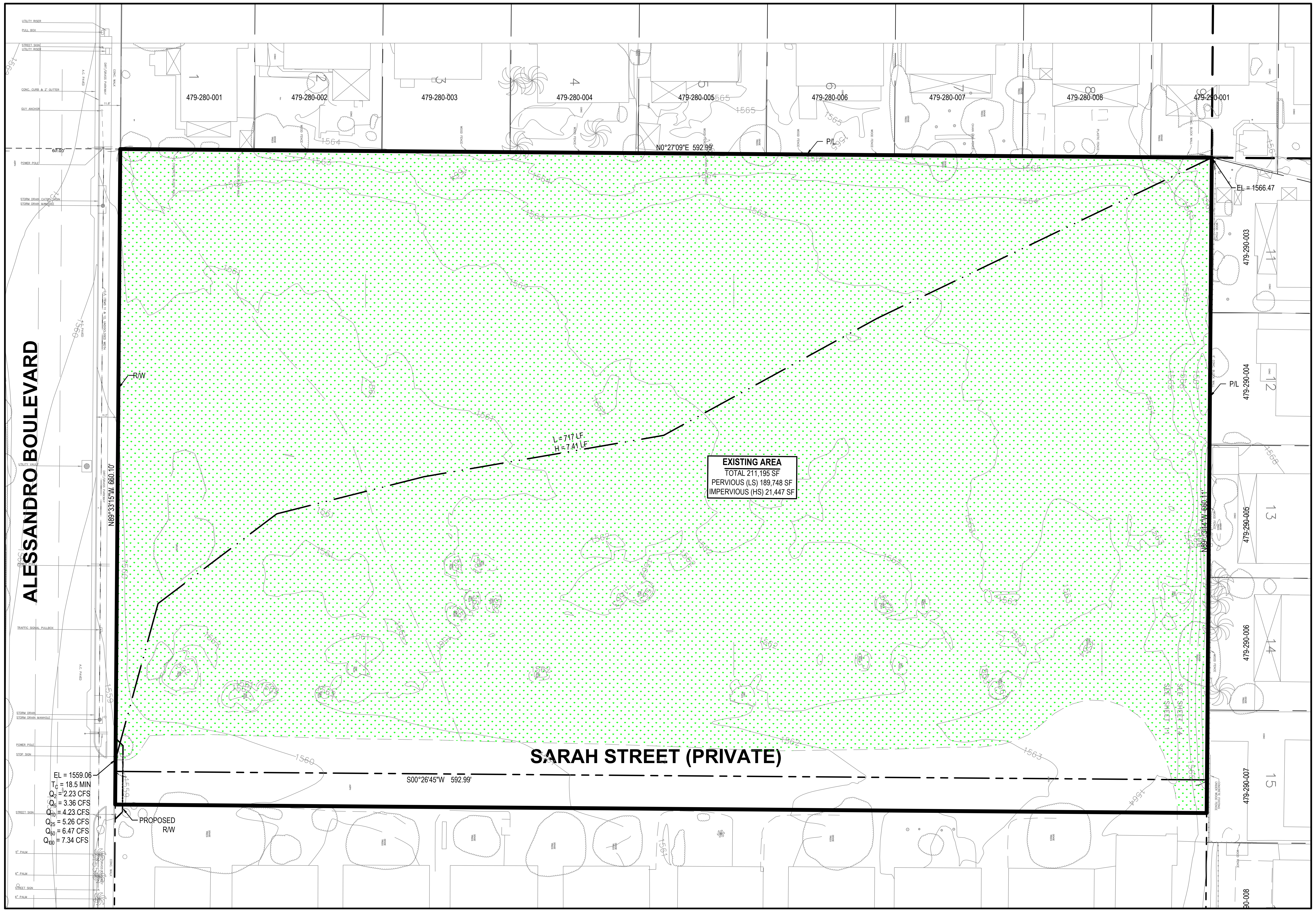
RUNOFF COEFFICIENT CURVES
SOIL GROUP-C
COVER TYPE-URBAN LANDSCAPING
AMC-II
(RUNOFF INDEX NUMBER 69)

PLATE D-5.3

Appendix C – Hydrology Maps

EXISTING HYDROLOGY MAP
VALLEY GARDENS

13989 MORENO ROSE PLACE,
MORENO VALLEY, CA 92553



EXISTING AREA
TOTAL 211,195 SF
PERVIOUS (LS) 189,748 SF
IMPERVIOUS (HS) 21,447 SF

EL = 1559.06
T_C = 18.5 MIN
Q_2 = 2.23 CFS
Q_5 = 3.36 CFS
Q_{10} = 4.23 CFS
Q_{25} = 5.26 CFS
Q_{50} = 6.47 CFS
Q_{100} = 7.34 CFS

LEGEND:

🟩	EXISTING PERVIOUS AREA
☐	EXISTING IMPERVIOUS AREA
━━━	DRAINAGE BOUNDARY
— — —	FLOW PATH

PEN21-0250 (LST21-0073)

30' 15' 0 30' 60'
SCALE: 1" = 30'

DIGALERT
DIAL BEFORE YOU DIG
TWO WORKING DAYS BEFORE YOU DIG
TOLL FREE 1-800-227-2600
A PUBLIC SERVICE BY UNDERGROUND SERVICE ALERT

REVISIONS

DATE	△	DESCRIPTION

BENCHMARK

DESCRIPTION:
485FT SOUTH OF C/L ALESSANDRO BLVD., 48FT EAST OF C/L PERRIS BLVD., 34FT NORTH OF ECR TO STRIP MALL, 76FT SOUTH OF BCR TO WALLGREENS. 1FT EAST OF SIDEWALK, SET 3" BRASS DISK IN CONC. FLUSH WITH SIDEWALK STAMPED LS 8136, M-79 RESET 2009.
ELEVATION 1563.092 FEET (NGVD 29).

BASIS OF BEARINGS

THE CENTERLINE OF ALESSANDRO BOULEVARD SHOWN AS N 89° 33' 15" W, ON TRACT MAP NO. 10056, AS FILED IN BOOK 102, PAGES 67 THROUGH 73 OF MAPS, RECORDS OF RIVERSIDE COUNTY, CALIFORNIA, WAS USED AS THE BASIS OF BEARINGS.

MAHIR WABER
No. C69050
Exp. 6/30/22

Mahir Waber

Waber Consultants INC
PLANNING CIVIL ENGINEERING SURVEYING
19210 S. VERMONT AVE., SUITE 115, GARDENA, CA 90248
P (424) 344-2464 F (562) 372-3282

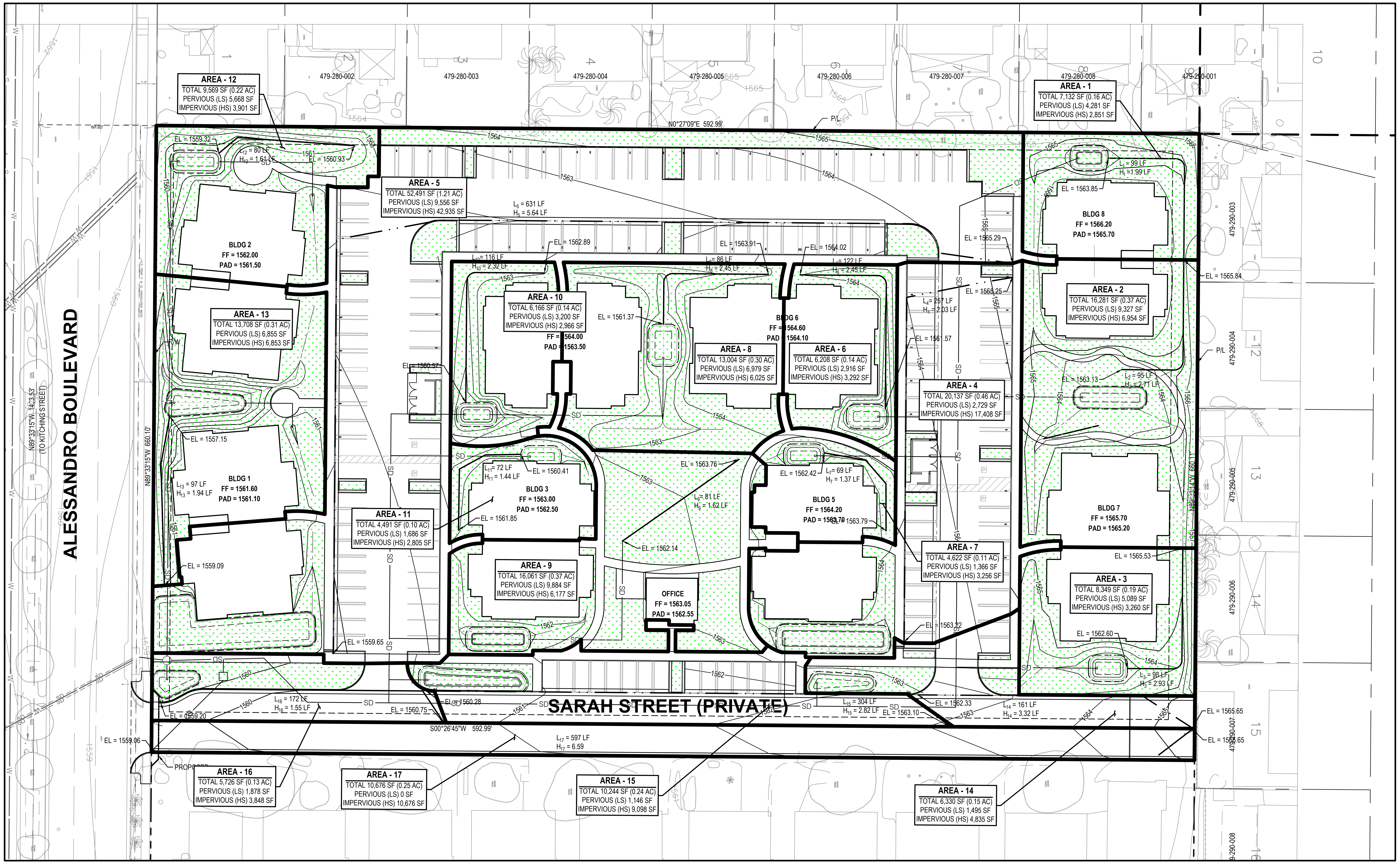
EXISTING HYDROLOGY MAP

VALLEY GARDENS
13989 MORENO ROSE PLACE, MORENO VALLEY, CA 92553

JOB NO. 21036
DATE: 10/21/2022
SHEET 1 OF 2 SHEETS

W:\21-036 VICTORY GARDENS PROJECT\DWG\21036HD01EX.dwg Oct 21, 2022 - 9:11am

PROPOSED HYDROLOGY MAP
VALLEY GARDENS
13989 MORENO ROSE PLACE,
MORENO VALLEY, CA 92553



LEGEND:

- 🟩 PROPOSED PERVIOUS AREA
- ☐ PROPOSED IMPERVIOUS AREA
- ━━ DRAINAGE BOUNDARY
- — · · — FLOW PATH

Subarea	2-year	5-year	10-year	25-year	50-year	100-year	Area (sf)	Area (ac)
Area 1	0.22	0.29	0.37	0.43	0.52	0.58	7,132	0.16
Area 2	0.50	0.66	0.83	0.97	1.18	1.31	16,281	0.37
Area 3	0.26	0.34	0.43	0.50	0.61	0.63	8,349	0.19
Area 4	0.53	0.72	0.87	1.04	1.22	1.36	20,137	0.46
Area 5	1.12	1.55	1.85	2.21	2.63	2.96	52,491	1.21
Area 6	0.19	0.25	0.32	0.37	0.45	0.50	6,208	0.14
Area 7	0.14	0.19	0.23	0.27	0.33	0.37	4,622	0.11
Area 8	0.40	0.53	0.66	0.77	0.93	1.04	13,004	0.40
Area 9	0.50	0.66	0.83	0.96	1.17	1.30	16,061	0.50
Area 10	0.19	0.25	0.32	0.37	0.45	0.50	6,166	0.19
Area 11	0.14	0.18	0.23	0.27	0.33	0.34	4,491	0.14
Area 12	0.30	0.39	0.49	0.57	0.70	0.78	9,569	0.30
Area 13	0.42	0.55	0.69	0.81	0.98	1.10	13,708	0.42
Area 14	0.16	0.22	0.26	0.32	0.37	0.42	6,330	0.16
Area 15	0.26	0.36	0.42	0.51	0.60	0.67	10,244	0.26
Area 16	0.16	0.22	0.26	0.31	0.37	0.41	5,726	0.16
Area 17	0.26	0.33	0.39	0.49	0.55	0.62	10,676	0.26
Total	5.74	7.70	9.45	11.16	13.38	14.88	211,195	4.85

PEN21-0250 (LST21-0073)

30' 15' 0 30' 60'
SCALE: 1" = 30'

DIGALERT
DIAL BEFORE YOU DIG
TWO WORKING DAYS BEFORE YOU DIG
TOLL FREE 1-800-227-2600
A PUBLIC SERVICE BY UNDERGROUND SERVICE ALERT

REVISIONS

DATE	△	DESCRIPTION

BENCHMARK

DESCRIPTION:
485FT SOUTH OF C/L ALESSANDRO BLVD., 48FT EAST OF C/L PERRIS BLVD., 34FT NORTH OF ECR TO STRIP MALL, 76FT SOUTH OF BCR TO WALLGREENS. 1FT EAST OF SIDEWALK, SET 3" BRASS DISK IN CONC. FLUSH WITH SIDEWALK STAMPED LS 8136, M-79 RESET 2009.
ELEVATION 1563.092 FEET (NGVD 29).

BASIS OF BEARINGS

THE CENTERLINE OF ALESSANDRO BOULEVARD SHOWN AS N 89° 33' 15" W, ON TRACT MAP NO. 10056, AS FILED IN BOOK 102, PAGES 67 THROUGH 73 OF MAPS, RECORDS OF RIVERSIDE COUNTY, CALIFORNIA, WAS USED AS THE BASIS OF BEARINGS.

MAHIR WABER
No. C69050
Exp. 6/30/22

Mahir Waber

Waber Consultants INC
PLANNING CIVIL ENGINEERING SURVEYING
19210 S. VERMONT AVE., SUITE 115, GARDENA, CA 90248
P (424) 344-2464 F (562) 372-3282

PROPOSED HYDROLOGY MAP

VALLEY GARDENS
13989 MORENO ROSE PLACE, MORENO VALLEY, CA 92553

JOB NO. 21036
DATE: 10/21/2022
SHEET 2 OF 2 SHEETS

W:\21-036 VICTORY GARDENS PROJECT\DWG\21036HD02PR.dwg Oct 21, 2022 - 9:11am

Appendix D – Soil Report

Geotechnical Engineering Investigation
Proposed Multi-Unit Residential Development
North of Alessandro Boulevard and
East of Flaming Arrow Drive
City of Moreno Valley, California

Tran Chung
39903 Camden Court
Temecula, California 92591

Project Number 22686-21
August 27, 2021

NorCal Engineering

NorCal Engineering
Soils and Geotechnical Consultants
10641 Humbolt Street Los Alamitos, CA 90720
(562) 799-9469 Fax (562) 799-9459

August 27, 2021 Project Number 22686-21

Tran Chung
39903 Camden Court
Temecula, California 92591

RE: **Geotechnical Engineering Investigation** - Proposed Multi-Unit Residential Development - Located North of Alessandro Boulevard and East of Flaming Arrow Drive, in the City of Moreno Valley, California

Dear Mr. Chung:

Pursuant to your request, this firm has performed a Geotechnical Engineering Investigation for the above referenced project in accordance with your approval of our proposal dated July 13, 2021. The purpose of this investigation is to evaluate the geotechnical conditions of the subject site and to provide recommendations for the proposed multi-unit residential development.

The scope of work included the following: 1) site reconnaissance; 2) subsurface geotechnical exploration and sampling; 3) laboratory testing; 4) soil infiltration testing; 5) engineering analysis of field and laboratory data; 5) preparation of a geotechnical engineering report. It is the opinion of this firm that the proposed development is feasible from a geotechnical standpoint provided that the recommendations presented in this report are followed in the design and construction of the project.

1.0 Project Description

It is proposed to construct a two-story, 67-unit residential development as shown on the attached Site Plan by Irwin Partners Architects dated August, 5 2021. Other improvements will include asphalt and/or concrete driveways, hardscape and landscaping. The proposed grading will consist of cuts on the order of a few feet with minor fills to achieve finished grade elevations. Final building plans shall be reviewed by this firm prior to submittal for city approval to determine the need for any additional study and revised recommendations pertinent to the proposed development, if necessary.

2.0 Site Description

The subject property is located along the north side of Alessandro Boulevard, east of Flaming Arrow Drive, in the City of Moreno Valley. The generally rectangular shaped parcel is elongated in a north to south direction. The site is relatively level with topography descending gradually from north to south on the order of a few feet. The site is currently vacant and covered in light vegetation.

3.0 Site Exploration

The field investigation consisted of the placement of ten (10) subsurface exploratory trenches by a backhoe to depths ranging between 5 and 16 feet below current ground elevations. The explorations were visually classified and logged by a field engineer with location of the subsurface explorations shown on the attached Site Plan. The exploratory borings revealed the existing earth materials to consist of fill and natural soil. Detailed descriptions of the subsurface conditions are listed on the boring logs in Appendix A. It should be noted that the transition from one soil type to another as shown on the borings logs is approximate and may in fact be a gradual transition. The soils encountered are described as follows:

Fill: A fill soil classifying as a brown, clayey SILT with some sand and occasional gravel, concrete and rootlets was encountered across the site to a depth of 1 to 1½ feet below ground surface. These soils were noted to be soft to medium stiff and dry.

Natural: An undisturbed native soil classifying as brown, silty sandy CLAY was encountered beneath the fill soils. The native soils were observed to be medium stiff to stiff and dry to damp.

The overall engineering characteristics of the earth material were relatively uniform with each excavation. No groundwater was encountered to the depths of our borings and no caving occurred.

4.0 Laboratory Tests

Relatively undisturbed samples of the subsurface soils were obtained to perform laboratory testing and analysis for direct shear, consolidation tests, and to determine in-place moisture/densities. These relatively undisturbed ring samples were obtained by driving a thin-walled steel sampler lined with one-inch-long brass rings with an inside diameter of 2.42 inches into the undisturbed soils. The sampler was driven a total of six inches into undisturbed soils. Bulk bag samples were obtained in the upper soils for expansion index tests and maximum density tests. All test results are included in Appendix B, unless otherwise noted.

4.1 **Field Moisture Content** (ASTM: D 2216) and the dry density of the ring samples were determined in the laboratory. This data is listed on the logs of explorations.

4.2 **Maximum Density tests** (ASTM: D 1557) were performed on typical samples of the upper soils. Results of these tests are shown on Table I.

4.3 **Expansion Index tests** (ASTM: D 4829) were performed on remolded samples of the upper soils to determine expansive characteristics. Results of these tests are provided on Table II.

4.4 **Atterberg Limits** (ASTM: D 4318) consisting of liquid limit, plastic limit and plasticity index were performed on representative soil samples. Results are shown on Table III.

4.5 **Corrosion tests** consisting of sulfate, pH, resistivity and chloride analysis to determine potential corrosive effects of soils on concrete and underground utilities. Test results are provided on Table IV.

4.6 **R-Value test** per California Test Method 301 was performed on a representative sample, which may be anticipated to be near subgrade to determine pavement design. Results are provided within the pavement design section of the report.

4.7 **Direct Shear tests** (ASTM: D 3080) were performed on undisturbed and/or remolded samples of the subsurface soils. The test is performed under saturated conditions at loads of 1,000 lbs./sq.ft., 2,000 lbs./sq.ft., and 3,000 lbs./sq.ft. with results shown on Plate A.

4.8 **Consolidation tests** (ASTM: D 2435) were performed on undisturbed samples to determine the differential and total settlement which may be anticipated based upon the proposed loads. Water was added to the samples at a surcharge of one KSF and the settlement curves are plotted on Plates B and C.

5.0 Seismicity Evaluation

The proposed development lies outside of any Alquist-Priolo Special Studies Zone and the potential for damage due to direct fault rupture is considered unlikely. The San Jacinto (Valley Segment) Fault is located approximately 7 kilometers from the site and is capable of producing a Magnitude 6.9 earthquake. Ground shaking originating from earthquakes along other active faults in the region is expected to induce lower horizontal accelerations due to smaller anticipated earthquakes and/or greater distances to other faults. The seismic design acceleration parameters are provided below and are based on the 2019 California Building Code (CBC) for the referenced project. The data was obtained from the American Society of Civil Engineers (ASCE) website, https://asce7hazardtool.online/ and is attached in Appendix C.

Seismic Design Acceleration Parameters

Latitude	33.918
Longitude	-117.221
Site Class	D
Risk Category	II
Mapped Spectral Response Acceleration	S_S = 1.653 S_1 = 0.644
Adjusted Maximum Acceleration	S_{MS} = 1.653
Design Spectral Response Acceleration Parameters	S_{DS} = 1.102
Peak Ground Acceleration	PGA_M = 0.77

Use of these values is dependent on requirements of ASCE 7-16, 11-4.8, Exception 2 that requires the value of the seismic response coefficient C_s be determined by Equation 12.8.2 for values of $T \leq 1.5T_s$ and taken as equal to 1.5 times the value computed in accordance with either 12.8-3 for $T_L \geq T \geq 1.5T_s$ or Equation 12.8-4 for $T > T_L$. Computations and verification of these conditions is referred to the structural engineer.

6.0 Liquefaction Evaluation

The site is expected to experience ground shaking and earthquake activity that is typical of the Southern California area. It is during severe shaking that loose, granular soils below the groundwater table can liquefy. Based on review of the *City of Moreno Valley Geological Faults and Liquefaction Map (September 22, 2016, revised May 2017)*, the site is *not* situated in an area of generalized liquefaction susceptibility. Thus, design of the proposed construction in conformance with the latest Building Code provisions for earthquake design is expected to provide mitigation of ground shaking hazards that are typical in Southern California.

7.0 Infiltration Characteristics

Infiltration tests within the site were performed to provide preliminary infiltration rates for the purpose of planning and design of an on-site water disposal system. The infiltration tests consisted of the double ring infiltration test per ASTM Method D 3385. The field infiltration rate was computed using a reduction factor – R_f based on the field measurements with our calculations given in Appendix D. Based upon the results of our testing, the soils encountered in the planned on-site drainage disposal system area exhibit the following infiltration rates.

Boring/Test No.	Depth	Soil Classification	Field Infiltration Rate	Design Rate
T-1	5'	Silty Sandy CLAY	0.58 in/hr	0.19 in/hr
T-2	7.5'	Silty SAND	0.64 in/hr	0.21 in/hr

Based on the results of our field testing, the subsurface soils encountered in the proposed on-site drainage disposal system shall utilize the design infiltration rates based on a safety factor of 3.0 or greater in compliance with the County of Riverside “Low Impact Development BMP Design” guidelines. All systems must meet the latest city and/or county specifications and the California Regional Water Quality Control Board (CRWQCB) requirements.

The infiltration rates at the depths tested indicate the stiff/dense soils encountered in our test locations *are not* suitable for storm water infiltration at the project site. The recommendations and conclusions contained in this report are based upon the soil conditions encountered in the test excavations.

8.0 Conclusions and Recommendations

Based upon our evaluations, the proposed development is acceptable from a geotechnical engineering standpoint. By following the recommendations and guidelines set forth in our report, the structures will be safe from excessive settlements under the anticipated design loadings and conditions. The proposed development shall meet all requirements of the City Building Ordinance and will not impose any adverse effect on existing adjacent structures.

The following recommendations are based upon soil conditions encountered in our field investigation; these near-surface soil conditions could vary across the site. Variations in the soil conditions may not become evident until the commencement of grading operations for the proposed development and revised recommendations from the soils engineer may be necessary based upon the conditions encountered.

It is recommended that site inspections are performed by a representative of this firm during all grading and construction of the development to verify the findings and recommendations documented in this report. The following sections present a discussion of geotechnical related requirements for specific design recommendations of different aspects of the project.

8.1 Site Grading Recommendations

All vegetation and demolition debris shall be removed and hauled prior to the start of grading operations. Existing vegetation shall not be mixed or disced into the soils. Any removed soils may be reutilized as compacted fill once any deleterious material or oversized materials (in excess of eight inches) is removed. Grading operations shall be performed in accordance with the attached "Specifications for Compacted Fill Operations".

8.1.1 Removal and Recompaction Recommendations

All disturbed soils and/or fill (about 1 to 1½ feet below ground surface) shall be removed to competent native material, the exposed surface scarified to a depth of 12 inches, brought to within 2% of optimum moisture content and compacted to a minimum of 90% of the laboratory standard (ASTM: D 1557) prior to placement of any additional compacted fill soils, foundations, slabs-on-grade and pavement. Grading shall extend a minimum of five horizontal feet outside the edges of foundations or equidistant to the depth of fill placed, whichever is greater.

It is possible that isolated areas of undiscovered fill not described in this report are present on site; if found, these areas should be treated as discussed earlier. A diligent search shall also be conducted during grading operations in an effort to uncover any underground structures, irrigation or utility lines. If encountered, these structures and lines shall be either removed or properly abandoned prior to the proposed construction.

Any imported fill material should be preferably soil similar to the upper soils encountered at the subject site. All soils shall be approved by this firm prior to importing at the site and will be subjected to additional laboratory testing to assure concurrence with the recommendations stated in this report.

If placement of slabs-on-grade and pavement is not completed immediately upon completion of grading operations, additional testing and grading of the areas may be necessary prior to continuation of construction operations. Likewise, if adverse weather conditions occur which may damage the subgrade soils, additional assessment by the soils engineer as to the suitability of the supporting soils may be needed.

Care should be taken to provide or maintain adequate lateral support for all adjacent improvements and structures at all times during the grading operations and construction phase. Adequate drainage away from the structures, pavement and slopes should be provided at all times.

8.1.2 Fill Blanket Recommendations

Due to the potential for differential settlement of foundations placed on compacted fill and medium stiff native materials, it is recommended that all foundations be underlain by a uniform compacted fill blanket at least two feet in thickness. This fill blanket shall extend a minimum of five horizontal feet outside the edges of foundations or equidistant to the depth of fill placed, whichever is greater.

8.2 Temporary Excavations

Temporary *unsurcharged* excavations in the existing site materials may be made at vertical inclinations up to 4 feet in height unless cohesionless soils are encountered. In areas where soils with little or no binder are encountered, where adverse geological conditions are exposed, or where excavations are adjacent to existing structures, shoring or flatter excavations may be required. The temporary cut slope gradients given above do not preclude local raveling and sloughing. Once finalized sections are made available, this firm shall review and provide stability calculations with updated excavation recommendations.

Temporary shoring design may utilize an active earth pressure of 25 pcf without any surcharge due to adjacent traffic, equipment or structures. The passive fluid pressures of 250 pcf may be doubled to 500 pcf for temporary design. Any drilled caissons will require to be cased due to the potential of caving. Shoring members should not be vibrated or driven due to the potential for damage to nearby improvements. All excavations shall be made in accordance with the requirements of the geotechnical engineer, CAL-OSHA and other public agencies having jurisdiction. Care should be taken to provide or maintain adequate lateral support for all adjacent improvements and structures at all times during the grading operations and construction phase.

8.3 Foundation Design

All foundations may be designed utilizing an allowable bearing capacity of 2,000 psf for a minimum embedded depth of 24 inches into approved engineered fill. The bearing value may be increased by 500 psf for each additional foot of depth in excess of the 24-inch minimum depth, up to 3,000 psf. A one-third increase may be used when considering short-term loading and seismic forces. All foundations shall be reinforced a minimum of one No. 4 bar, top and bottom. A representative of this firm shall inspect all foundation excavations prior to pouring concrete.

8.4 Settlement Analysis

Resultant pressure curves for the consolidation tests are shown on Plates B and C. Computations utilizing these curves and the recommended allowable soil bearing capacities reveal that the foundations will experience settlements on the order of ¾ inch and differential settlements of less than ¼ inch.

8.5 Lateral Resistance

The following values may be utilized in resisting lateral loads imposed on the structure. Requirements of the California Building Code should be adhered to when the coefficient of friction and passive pressures are combined.

Coefficient of Friction - 0.35
Equivalent Passive Fluid Pressure = 200 lbs./cu.ft.
Maximum Passive Pressure = 2,000 lbs./cu.ft.

The passive pressure recommendations are valid only for approved compacted fill soils or competent native materials.

8.6 Retaining Wall Design Parameters

Active earth pressures against retaining walls will be equal to the pressures developed by the following fluid densities. These values are for **granular backfill material** placed behind the walls at various ground slopes above the walls.

Surface Slope of Retained Materials (Horizontal to Vertical	Equivalent Fluid Density (lb./cu.ft.)
Level	30
5 to 1	35
4 to 1	38
3 to 1	40
2 to 1	45

Any applicable short-term construction surcharges and seismic forces should be added to the above lateral pressure values. An equivalent fluid pressure of 45 pcf may be utilized for the restrained wall condition with a level grade behind the wall.

The seismic-induced lateral soil pressure for walls greater than 6 feet may be computed using a triangular pressure distribution with the maximum value at the top of the wall. The maximum lateral pressure of (20 pcf) H where H is the height of the retained soils above the wall footing should be used in final design of retaining walls. Sliding resistance values and passive fluid pressure values may be increased by 1/3 during short-term wind and seismic loading conditions.

All walls shall be waterproofed as needed and protected from hydrostatic pressure by a reliable permanent subdrain system. The subsurface drainage system shall consist of a 4-inch diameter perforated PVC pipe encased with gravel and wrapped with filter fabric. The granular backfill to be utilized immediately adjacent to retaining walls shall consist of an approved granular soil with a sand equivalency greater than 30. This backfill zone of free draining material shall consist of a wedge beginning a minimum of one horizontal foot from the base of the wall extending upward at an inclination of no less than ¾ to 1 (horizontal to vertical).

8.7 <u>Slab Design</u>

All slabs shall be a minimum of four inches in thickness reinforced a minimum of No. 3 bars, sixteen inches in each direction and placed on approved subgrade soils. The subgrade soils shall be moisture conditioned 3% over optimum moisture levels in the upper eighteen inches. All concrete slabs for hardscape and driveway areas situated near existing street levels shall be a minimum of four inches in thickness and placed on approved subgrade soils. The subgrade soils shall be moisture conditioned over optimum moisture levels in the upper foot.

A vapor retarder should be utilized in areas which would be sensitive to the infiltration of moisture. This retarder shall meet requirements of ASTM E 96, *Water Vapor Transmission of Materials* and ASTM E 1745, *Standard Specification for Water Vapor Retarders used in Contact with Soil or Granular Fill Under Concrete Slabs.* The vapor retarder shall be installed in accordance with procedures stated in ASTM E 1643, *Standard practice for Installation of Water Vapor Retarders used in Contact with Earth or Granular Fill Under Concrete Slabs.*

The moisture retarder may be placed directly upon compacted subgrade soils conditioned to near optimum moisture levels, although one to two inches of sand beneath the membrane is desirable. The subgrade upon which the retarder is placed shall be smooth and free of rocks, gravel or other protrusions which may damage the retarder. Use of sand above the retarder is under the purview of the structural engineer; if sand is used over the retarder, it should be placed in a dry condition.

8.8 Pavement Section Design

The table below provides a preliminary pavement design based upon a R-Value of 33 for the subgrade soils for the proposed pavement areas. Final pavement design may need to be based on R-Value testing of the subgrade soils near the conclusion of site grading to assure that these soils are consistent with those assumed in this preliminary design.

Type of Traffic	Traffic Index	Asphalt (in.)	Base Material (in.)
Automobile Parking Stalls	4.0	3.0	4.0
Light Vehicle Traffic Areas	5.5	3.5	6.5

Any concrete slab-on-grade in pavement areas shall be a minimum of six inches in thickness reinforced and placed on approved subgrade soils. All pavement areas shall have positive drainage toward an approved outlet from the site. Drain lines behind curbs and/or adjacent to landscape areas should be considered by client and the appropriate design engineers to prevent water from infiltrating beneath pavement. If such infiltration occurs, damage to pavement, curbs and flow lines, especially on sites with expansive soils, may occur during the life of the project.

Any approved base material shall consist of a Class II aggregate or equivalent and should be compacted to a minimum of 95% relative compaction. All pavement materials shall conform to the requirements set forth by the City of Riverside. The base material; and asphaltic concrete should be tested prior to delivery to the site and during placement to determine conformance with the project specifications. A pavement engineer shall designate the specific asphalt mix design to meet the required project specifications.

8.9 Utility Trench and Excavation Backfill

Trenches from installation of utility lines and other excavations may be backfilled with on-site soils or approved imported soils compacted to a minimum of 90% relative compaction. All utility lines shall be properly bedded with clean sand having a sand equivalency rating of 30 or more. This bedding material shall be thoroughly water jetted around the pipe structure prior to placement of compacted backfill soils.

8.10 Corrosion Design Criteria

Representative samples of the surficial soils, typical of the subgrade soils expected to be encountered within foundation excavations and underground utilities were tested for corrosion potential. The minimum resistivity value obtained for the samples tested is representative of an environment that may be severely corrosive to metals. The soil pH value was considered to be neutral and may not have a significant effect on soil corrosivity.

Consideration should be given to corrosion protection systems for buried metal such as protective coatings, wrappings or the use of PVC where permitted by local building codes. According to Table 4.3.1 of ACI 318 Building Code and Commentary, these contents revealed negligible sulfate concentrations. Therefore, a Type II cement according to latest CBC specifications may be utilized for building foundations at this time. It is recommended that additional sulfate tests be performed at the completion of site grading to assure that the as graded conditions are consistent with the recommendations stated in this design. Corrosion test results may be found on the attached Table III.

8.11 Expansive Soil

The upper on-site soils are low un expansion potential (EI 21-50). When soils have an expansion index (EI) of 20 or more, special attention should be given to the project design and maintenance. The attached *Expansive Soil Guidelines* should be reviewed by the engineers, architects, owner, maintenance personnel and other interested parties and considered during the design of the project and future property maintenance. Expansion test results may be found on the attached Table II.

9.0 Closure

The recommendations and conclusions contained in this report are based upon the soil conditions uncovered in our test excavations. No warranty of the soil condition between our excavations is implied. NorCal Engineering should be notified for possible further recommendations if unexpected to unfavorable conditions are encountered during construction phase. It is the responsibility of the owner to ensure that all information within this report is submitted to the Architect and appropriate Engineers for the project.

This firm should have the opportunity to review the final plans (72 hours required) to verify that all our recommendations are incorporated. This report and all conclusions are subject to the review of the controlling authorities for the project.

A preconstruction conference should be held between the developer, general contractor, grading contractor, city inspector, architect, and soil engineer to clarify any questions relating to the grading operations and subsequent construction. Our representative should be present during the grading operations and construction phase to certify that such recommendations are complied within the field.

This geotechnical investigation has been conducted in a manner consistent with the level of care and skill exercised by members of our profession currently practicing under similar conditions in the Southern California area. No other warranty, expressed or implied is made.

We appreciate this opportunity to be of service to you. If you have any further questions, please do not hesitate to contact the undersigned.

Respectfully submitted,
NORCAL ENGINEERING

Keith D Tucker

REGISTERED PROFESSIONAL ENGINEER
KEITH D. TUCKER
No. 841
Exp. 12/31/2022
GEOTECHNICAL
STATE OF CALIFORNIA

Keith D. Tucker
Project Engineer
R.G.E. 841

MB

Mike Barone
Project Manager

SPECIFICATIONS FOR PLACEMENT OF COMPACTED FILL

Excavation

Any existing low-density soils and/or saturated soils shall be removed to competent natural soil under the inspection of the Geotechnical Engineering Firm. After the exposed surface has been cleansed of debris and/or vegetation, it shall be scarified until it is uniform in consistency, brought to the proper moisture content and compacted to a minimum of 90% relative compaction (in accordance with ASTM: D 1557).

In any area where a transition between fill and native soil or between bedrock and soil are encountered, additional excavation beneath foundations and slabs will be necessary in order to provide uniform support and avoid differential settlement of the structure.

Material for Fill

The on-site soils or approved import soils may be utilized for the compacted fill provided they are free of any deleterious materials and shall not contain any rocks, brick, asphaltic concrete, concrete or other hard materials greater than eight inches in maximum dimensions. Any import soil must be approved by the Geotechnical Engineering firm a minimum of 72 hours prior to importation of site.

Placement of Compacted Fill Soils

The approved fill soils shall be placed in layers not excess of six inches in thickness. Each lift shall be uniform in thickness and thoroughly blended. The fill soils shall be brought to within 2% of the optimum moisture content, unless otherwise specified by the Geotechnical Engineering firm. Each lift shall be compacted to a minimum of 90% relative compaction (in accordance with ASTM: D 1557) and approved prior to the placement of the next layer of soil. Compaction tests shall be obtained at the discretion of the Geotechnical Engineering firm but to a minimum of one test for every 500 cubic yards placed and/or for every 2 feet of compacted fill placed.

The minimum relative compaction shall be obtained in accordance with accepted methods in the construction industry. The final grade of the structural areas shall be in a dense and smooth condition prior to placement of slabs-on-grade or pavement areas. No fill soils shall be placed, spread or compacted during unfavorable weather conditions. When the grading is interrupted by heavy rains, compaction operations shall not be resumed until approved by the Geotechnical Engineering firm.

Grading Observations

The controlling governmental agencies should be notified prior to commencement of any grading operations. This firm recommends that the grading operations be conducted under the observation of a Geotechnical Engineering firm as deemed necessary. A 24-hour notice must be provided to this firm prior to the time of our initial inspection.

Observation shall include the clearing and grubbing operations to assure that all unsuitable materials have been properly removed; approve the exposed subgrade in areas to receive fill and in areas where excavation has resulted in the desired finished grade and designate areas of overexcavation; and perform field compaction tests to determine relative compaction achieved during fill placement. In addition, all foundation excavations shall be observed by the Geotechnical Engineering firm to confirm that appropriate bearing materials are present at the design grades and recommend any modifications to construct footings.

EXPANSIVE SOIL GUIDELINES

The following expansive soil guidelines are provided for your project. The intent of these guidelines is to inform you, the client, of the importance of proper design and maintenance of projects supported on expansive soils. ***You, as the owner or other interested party, should be warned that you have a duty to provide the information contained in the soil report including these guidelines to your design engineers, architects, landscapers and other design parties in order to enable them to provide a design that takes into consideration expansive soils.***

In addition, you should provide the soil report with these guidelines to any property manager, lessee, property purchaser or other interested party that will have or assume the responsibility of maintaining the development in the future.

Expansive soils are fine-grained silts and clays which are subject to swelling and contracting. The amount of this swelling and contracting is subject to the amount of fine-grained clay materials present in the soils and the amount of moisture either introduced or extracted from the soils. Expansive soils are divided into five categories ranging from “very low” to “very high”. Expansion indices are assigned to each classification and are included in the laboratory testing section of this report. *If the expansion index of the soils on your site, as stated in this report, is 21 or higher, you have expansive soils.* The classifications of expansive soils are as follows:

Classification of Expansive Soil*

Expansion Index	Potential Expansion
0-20	Very Low
21-50	Low
51-90	Medium
91-130	High
Above 130	Very High

*From Table 18A-I-B of California Building Code (1988)

When expansive soils are compacted during site grading operations, care is taken to place the materials at or slightly above optimum moisture levels and perform proper compaction operations. Any subsequent excessive wetting and/or drying of expansive soils will cause the soil materials to expand and/or contract. These actions are likely to cause distress of foundations, structures, slabs-on-grade, sidewalks and pavement over the life of the structure. ***It is therefore imperative that even after construction of improvements, the moisture contents are maintained at relatively constant levels, allowing neither excessive wetting or drying of soils.***

Evidence of excessive wetting of expansive soils may be seen in concrete slabs, both interior and exterior. Slabs may lift at construction joints producing a trip hazard or may crack from the pressure of soil expansion. Wet clays in foundation areas may result in lifting of the structure causing difficulty in the opening and closing of doors and windows, as well as cracking in exterior and interior wall surfaces. In extreme wetting of soils to depth, settlement of the structure may eventually result. Excessive wetting of soils in landscape areas adjacent to concrete or asphaltic pavement areas may also result in expansion of soils beneath pavement and resultant distress to the pavement surface.

Excessive drying of expansive soils is initially evidenced by cracking in the surface of the soils due to contraction. Settlement of structures and on-grade slabs may also eventually result along with problems in the operation of doors and windows.

Projects located in areas of expansive clay soils will be subject to more movement and "hairline" cracking of walls and slabs than similar projects situated on non-expansive sandy soils. There are, however, measures that developers and property owners may take to reduce the amount of movement over the life the development. The following guidelines are provided to assist you in both design and maintenance of projects on expansive soils:

- Drainage away from structures and pavement is essential to prevent excessive wetting of expansive soils. Grades should be designed to the latest building code and maintained to allow flow of irrigation and rain water to approved drainage devices or to the street. Any “ponding” of water adjacent to buildings, slabs and pavement after rains is evidence of poor drainage; the installation of drainage devices or regrading of the area may be required to assure proper drainage. Installation of rain gutters is also recommended to control the introduction of moisture next to buildings. Gutters should discharge into a drainage device or onto pavement which drains to roadways.

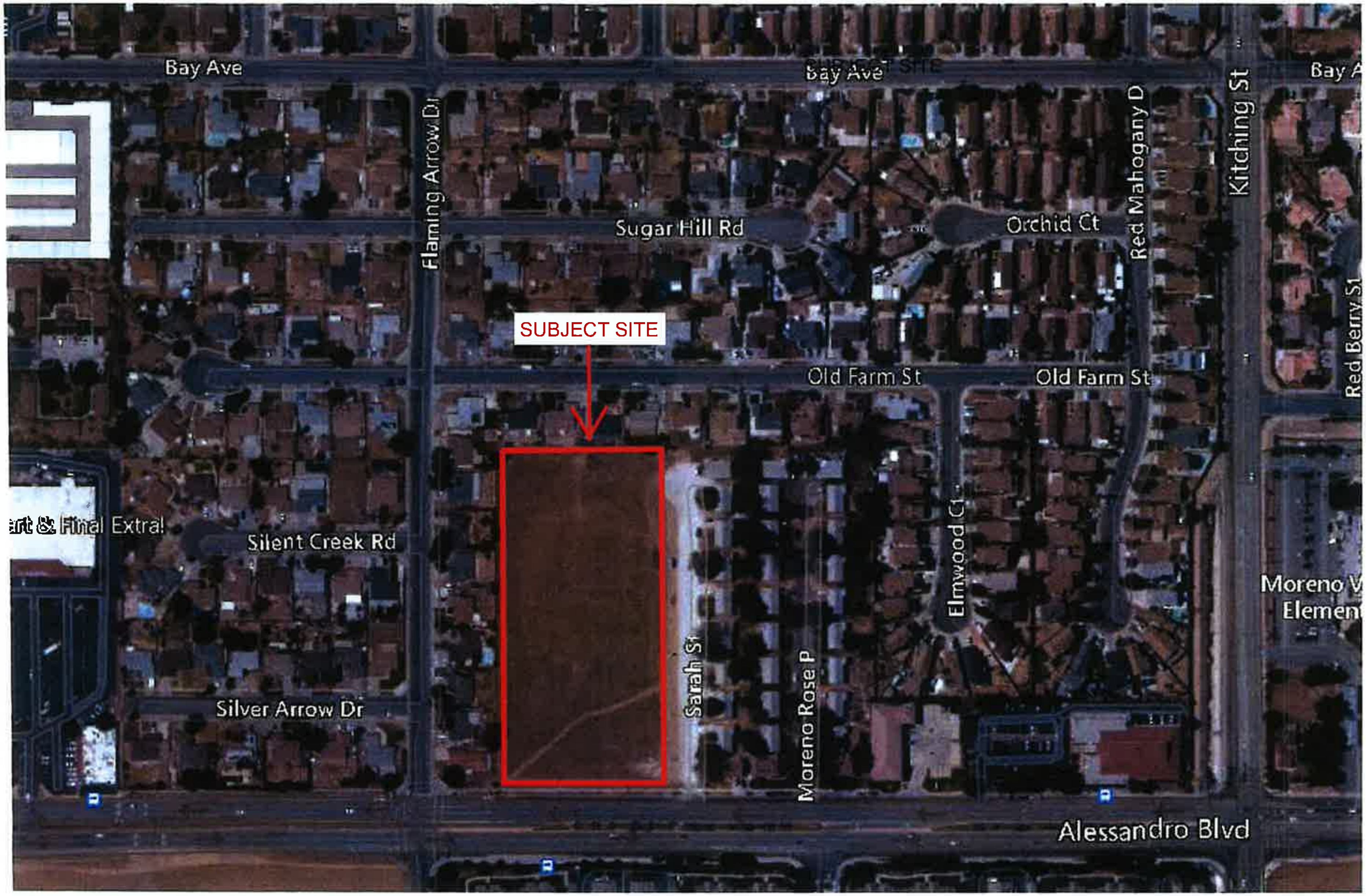
- Irrigation should be strictly controlled around building foundations, slabs and pavement and may need to be adjusted depending upon season. This control is essential to maintain a relatively uniform moisture content in the expansive soils and to prevent swelling and contracting. Over-watering adjacent to improvements may result in damage to those improvements. NorCal Engineering makes no specific recommendations regarding landscape irrigation schedules.

- Planting schemes for landscaping around structures and pavement should be analyzed carefully. Plants (including sod) requiring high amounts of water may result in excessive wetting of soils. Trees and large shrubs may actually extract moisture from the expansive soils, thus causing contraction of the fine-grained soils.

- Thickened edges on exterior slabs will assist in keeping excessive moisture from entering directly beneath the concrete. A six-inch thick or greater deepened edge on slabs may be considered. Underlying interior and exterior slabs with 6 to 12 inches or more of non-expansive soils and providing presaturation of the underlying clayey soils as recommended in the soil report will improve the overall performance of on-grade slabs.

- Increase the amount of steel reinforcing in concrete slabs, foundations and other structures to resist the forces of expansive soils. The precise amount of reinforcing should be determined by the appropriate design engineers and/or architects.

- Recommendations of the soil report should always be followed in the development of the project. Any recommendations regarding presaturation of the upper subgrade soils in slab areas should be performed in the field and verified by the Soil Engineer.



NorCal Engineering
SOILS AND GEOTECHNICAL CONSULTANTS

TRAN CHUNG

PROJECT: 22686-21	DATE: AUGUST 2021

VICINITY MAP

NorCal Engineering
SOILS AND GEOTECHNICAL CONSULTANTS
TRAN CHUNG

PROJECT 22686-21 **DATE** AUGUST 2021

SITE PLAN

List of Appendices
(in order of appearance)

Appendix A – Log of Excavations

Log of Borings T-1 to T-10

Appendix B – Laboratory Tests

Table I – Maximum Dry Density
Table II – Expansion
Table III – Atterberg Limits
Table IV - Corrosion
Plate A – Direct Shear
Plates B and C - Consolidation

Appendix C Seismic Design

ASCE Seismic Hazards Report

Moreno Valley Geological Faults and Liquefaction Map

Appendix D – Soil Infiltration

Soil Infiltration Data

Appendix A
Log of Excavations

Tran Chung
22686-21

Log of Trench T-1

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology		Material Description	Samples		Laboratory		
				Type	Blow Counts	Moisture	Dry Density	Fines Content %
0	▨	GWT not encountered	FILL Clayey silty SAND with occasional gravel, concrete Brown, medium stiff, dry					
	▨		NATURAL Silty sandy CLAY Brown, stiff, damp					
5			Trench completed at depth of 5'					
10								
15								
20								
25								
30								
35								

NorCal Engineering

Tran Chung
22686-21

Log of Trench T-2

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology		Material Description	Samples		Laboratory		
				Type	Blow Counts	Moisture	Dry Density	Fines Content %
0	▨	GWT not encountered	FILL Clayey silty SAND with occasional rootlets Brown, medium stiff, dry					
	▨		NATURAL Silty sandy CLAY Brown, stiff, damp					
5								
	▦		Silty (fine to coarse grained) SAND with occasional gravel Brown, dense, moist					
10			Trench completed at depth of 10'					
15								
20								
25								
30								
35								

NorCal Engineering

Tran Chung
22686-21

Log of Trench T-3

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

GWT not encountered

Depth (feet)	Material Description	Samples: Type	Samples: Blow Counts	Laboratory: Moisture	Laboratory: Dry Density	Laboratory: Fines Content %
0	FILL Clayey silty SAND with occasional gravel, rootlets Brown, medium stiff, dry	☒				
	NATURAL Silty sandy CLAY Brown, medium stiff, damp	■		4.1	117.1	
5		■		6.0	111.0	
10		■		6.5	92.8	
	Silty (fine to coarse grained) SAND with occasional gravel Brown, dense, moist					
	Silty CLAY Brown, stiff, moist					
15		■		11.9	105.5	
	Trench completed at depth of 16'					
20						
25						
30						
35						

SuperLog CivilTech Software, USA www.civiltech.com File: C:\Superlog4\PROJECT\22686-21.log Date: 8/24/2021

NorCal Engineering

Tran Chung
22686-21

Log of Trench T-4

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology		Material Description	Samples		Laboratory		
				Type	Blow Counts	Moisture	Dry Density	Fines Content %
0	▨	GWT not encountered	FILL Clayey silty SAND with occasional rootlets Brown, soft, dry	■		2.0	108.1	
	▨		NATURAL Silty sandy CLAY Brown, medium stiff, dry to damp					
5								
10			Trench completed at depth of 10'					
15								
20								
25								
30								
35								

NorCal Engineering

Tran Chung
22686-21

Log of Trench T-5

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith- ology		Material Description	Samples		Laboratory		
				Type	Blow Counts	Moisture	Dry Density	Fines Content %
0	▨	GWT not encountered	FILL Clayey silty SAND with occasional gravel, rootlets Brown, soft, dry					
5	▨		NATURAL Silty sandy CLAY Brown, stiff, damp	■		3.4	111.3	
10	▨		Sandy CLAY Brown, stiff, damp	■		8.6	106.6	
	▨		Silty CLAY Brown, stiff, damp Trench completed at depth of 12.5'	▯		8.5		
15								
20								
25								
30								
35								

Tran Chung
22686-21

Log of Trench T-6

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology	Material Description	Samples: Type	Samples: Blow Counts	Laboratory: Moisture	Laboratory: Dry Density	Laboratory: Fines Content %
0		FILL Clayey silty SAND with occasional gravel, concrete Brown, soft, dry	■		6.2	111.9	
		NATURAL Silty sandy CLAY Brown, stiff, damp					
5			■		5.5	117.9	
10		Trench completed at depth of 10'					

GWT not encountered

Tran Chung
22686-21

Log of Trench T-7

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology		Material Description	Samples: Type	Samples: Blow Counts	Laboratory: Moisture	Laboratory: Dry Density	Laboratory: Fines Content %
0	▨	GWT not encountered	FILL Clayey silty SAND with occasional rootlets Brown, soft, dry					
	▨		NATURAL Silty sandy CLAY Brown, stiff, damp					
5				■		3.6	114.3	
10	▨		Silty (fine to coarse grained) SAND with occasional gravel Brown, dense, damp					
			Trench completed at depth of 11'					
15								
20								
25								
30								
35								

NorCal Engineering

Tran Chung

22686-21

Log of Trench T-8

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology	Material Description	Samples: Type	Samples: Blow Counts	Laboratory: Moisture	Laboratory: Dry Density	Laboratory: Fines Content %
0		FILL Clayey silty SAND with occasional gravel, rootlets Brown, medium stiff, dry					
		NATURAL Silty sandy CLAY Brown, stiff, damp	■		4.9	114.3	
5		Trench completed at depth of 5'					
10							
15							
20							
25							
30							
35							

Tran Chung
22686-21

Log of Trench T-9

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith- ology	Material Description	Samples: Type	Samples: Blow Counts	Laboratory: Moisture	Laboratory: Dry Density	Laboratory: Fines Content %
0	GWT not encountered	FILL Clayey silty SAND with occasional gravel Brown, medium stiff, dry					
		NATURAL Silty sandy CLAY Brown, medium stiff, damp					
5			■		4.4	111.8	
10		Silty (fine to coarse grained) SAND with occasional gravel Brown, dense, damp					
		Trench completed at depth of 10'					
15							
20							
25							
30							
35							

SuperLog CivilTech Software, USA www.civiltech.com File: C:\Superlog4\PROJECT\22686-21.log Date: 8/24/2021

NorCal Engineering

Tran Chung
22686-21

Log of Trench T-10

Boring Location: Alessandro & Flaming Arrow, Moreno Valle

Date of Drilling: 7/23/21

Groundwater Depth: None Encountered

Drilling Method: Backhoe

Hammer Weight:

Drop:

Surface Elevation: Not Measured

Depth (feet)	Lith-ology		Material Description	Samples: Type	Samples: Blow Counts	Laboratory: Moisture	Laboratory: Dry Density	Laboratory: Fines Content %
0	▨	GWT not encountered	FILL Clayey silty SAND with occasional rootlets Brown, medium stiff, dry	■		1.3	116.7	
	▨		NATURAL Silty sandy CLAY Brown, stiff, dry to damp	■		4.7	114.6	
5			Trench completed at depth of 5'					
10								
15								
20								
25								
30								
35								

SuperLog CivilTech Software, USA www.civiltech.com File: C:\Superlog4\PROJECT\22686-21.log Date: 8/24/2021

NorCal Engineering

Appendix B
Laboratory Tests

TABLE I
MAXIMUM DENSITY TESTS

Sample	Classification	Optimum Moisture (%)	Maximum Dry Density (lbs/cu.ft)
T3 @ 1'	Silty Sandy CLAY	10.5	128.0

TABLE II
EXPANSION TESTS

Sample	Classification	Expansion Index
T3 @ 1'	Silty Sandy CLAY	33

TABLE III
ATTERBERG LIMITS

Sample	Liquid Limit	Plastic Limit	Plasticity Index
T-3 @ 5'	18	17	1
T-3 @ 10'	26	17	9

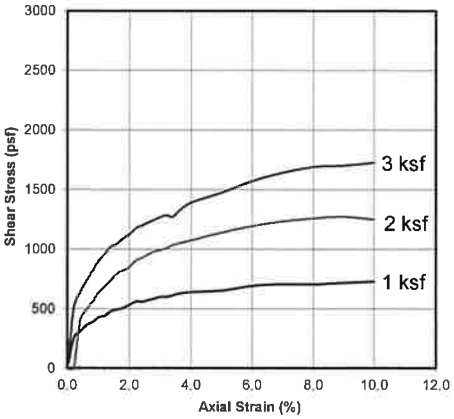
TABLE IV
CORROSION TESTS

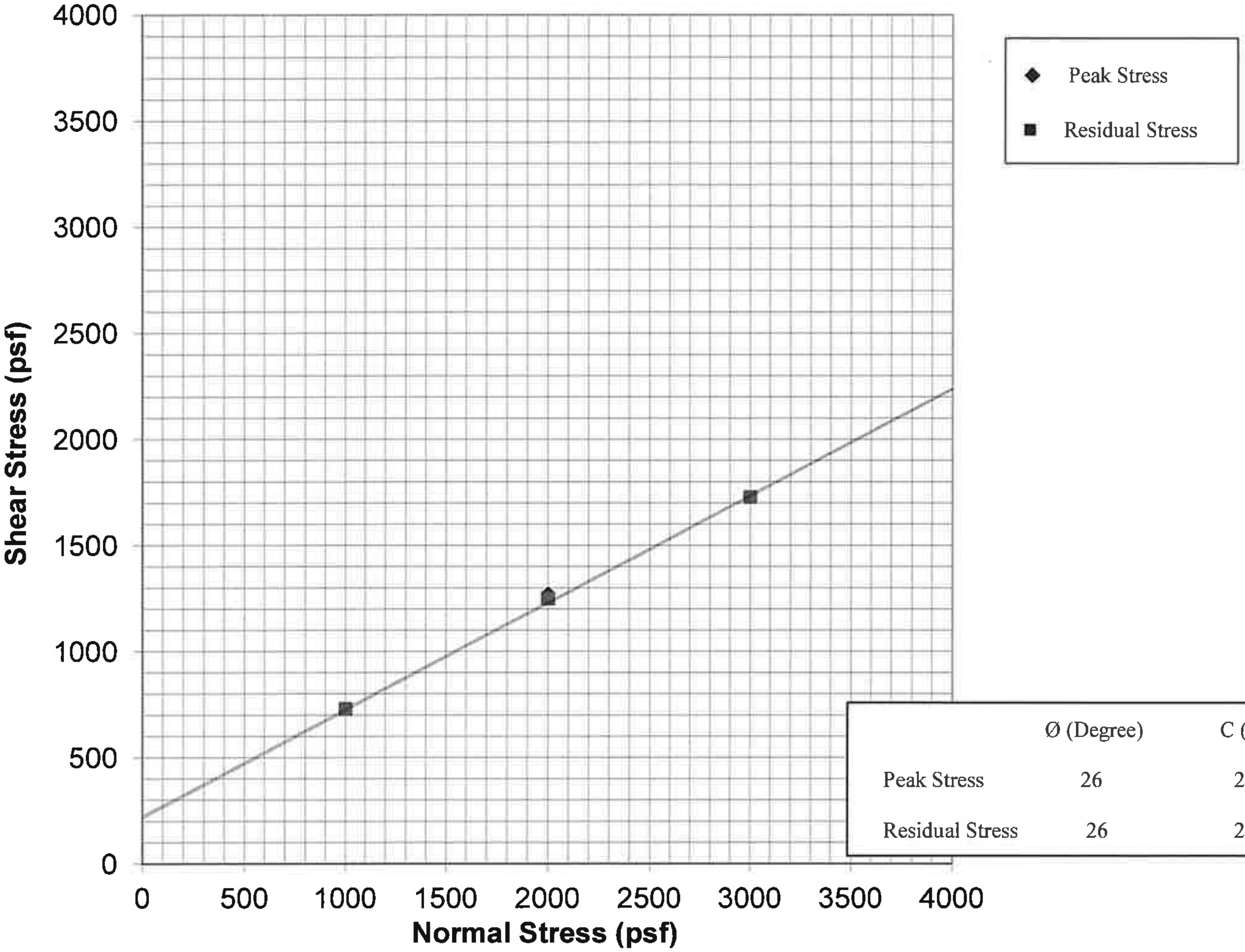
Sample	pH	Electrical Resistivity	Sulfate (%)	Chloride (ppm)
T-3 @ 1'	7.0	15,320	N.D.	111

% by weight
ppm – mg/kg
N.D. = Non-Detect

Sample No. T6@2'
Sample Type: Undisturbed/Saturated
Soil Description: Silty Sandy Clay

		1	2	3
Normal Stress	(psf)	1000	2000	3000
Peak Stress	(psf)	730	1273	1728
Displacement	(in)	0.250	0.225	0.250
Residual Stress	(psf)	730	1250	1728
Displacement	(in.)	0.250	0.250	0.250
In Situ Dry Density	(pcf)	111.9	111.9	111.9
In Situ Water Content	(%)	6.2	6.2	6.2
Saturated Water Content	(%)	18.7	18.7	18.7
Strain Rate	(in/min)	0.020	0.020	0.020





	Ø (Degree)	C (psf)
Peak Stress	26	225
Residual Stress	26	215

NorCal Engineering

SOILS AND GEOTECHNICAL CONSULTANTS

Tran Chung

PROJECT NUMBER: 22686-21 DATE: 8/17/2021

DIRECT SHEAR TEST

ASTM D3080

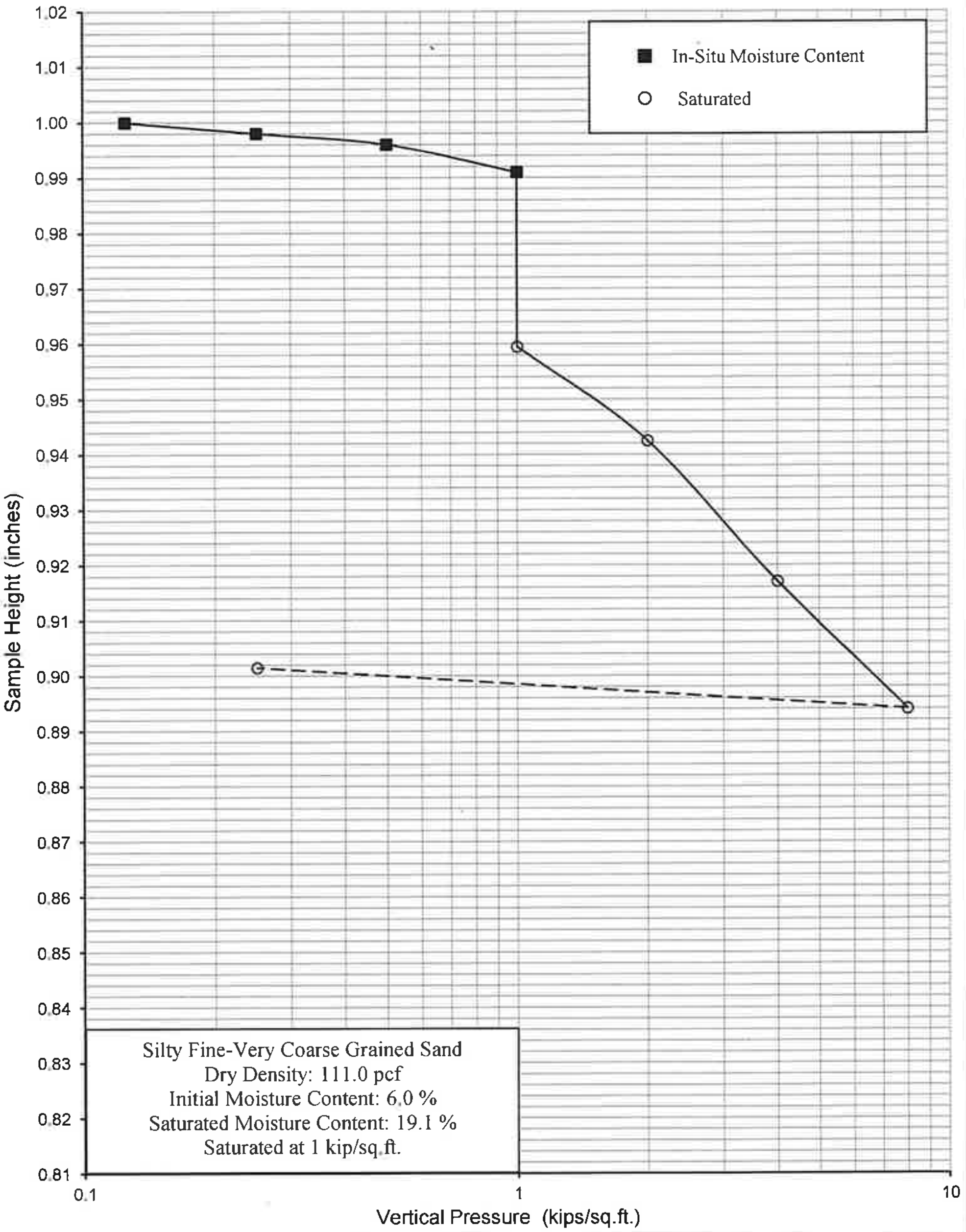
Plate A

Sample No. T3 | Depth 5' | Date 8/17/2021

Vertical Pressure (kips/sq.ft.)	Sample Height (inches)	Consolidation (percent)
0.125	1.0000	0.0
0.25	0.9980	0.2
0.5	0.9960	0.4
1	0.9910	0.9
1	0.9595	4.1
2	0.9425	5.8
4	0.9170	8.3
8	0.8940	10.6
0.25	0.9015	9.9

Saturated

Date Tested: 8/12/2021
Sample: T3
Depth: 5'



Silty Fine-Very Coarse Grained Sand
Dry Density: 111.0 pcf
Initial Moisture Content: 6.0 %
Saturated Moisture Content: 19.1 %
Saturated at 1 kip/sq.ft.

NorCal Engineering

SOILS AND GEOTECHNICAL CONSULTANTS

Tran Chung

PROJECT NUMBER: 22686-21 DATE: 8/17/2021

CONSOLIDATION TEST

ASTM D2435

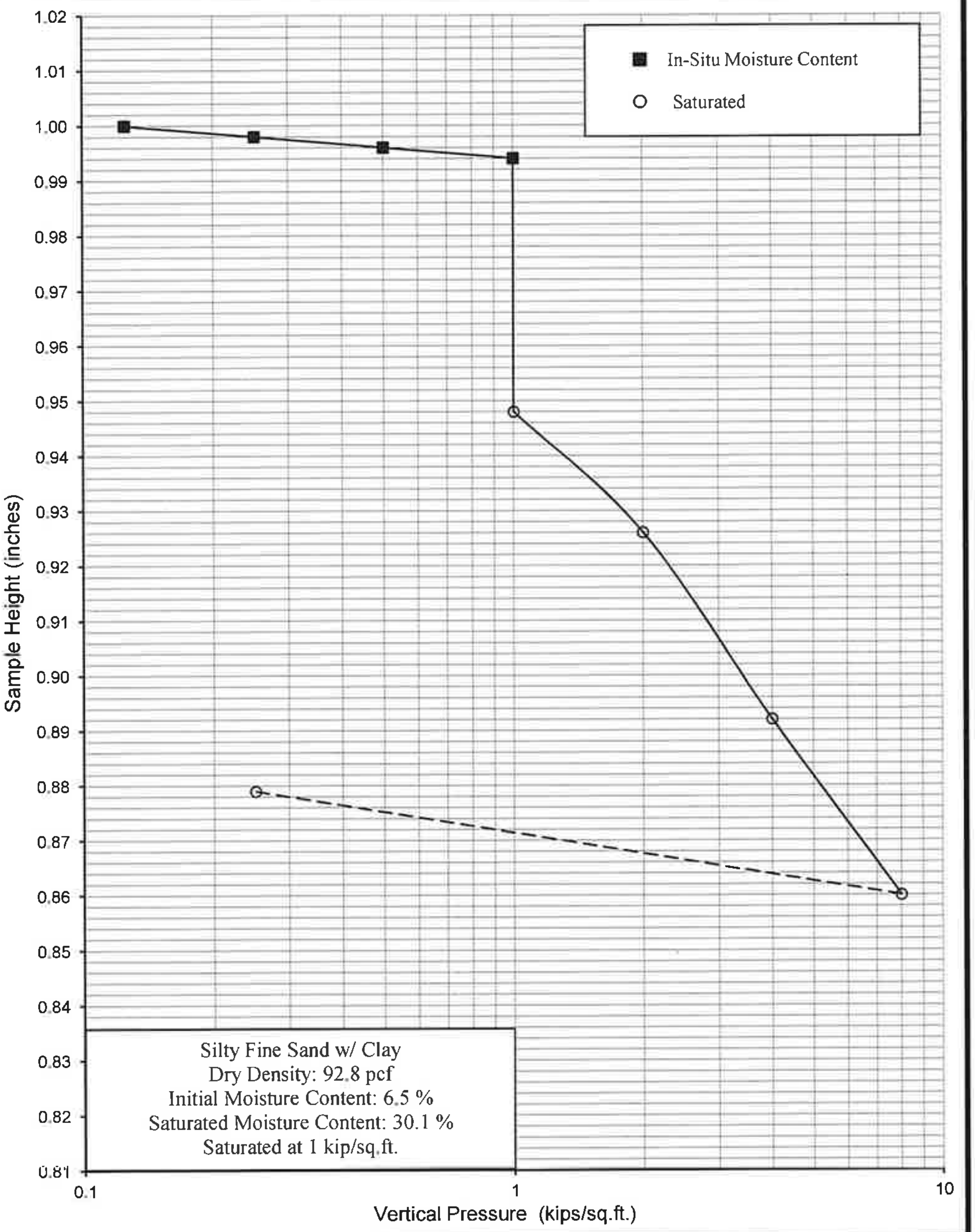
Plate B

Sample No. T3 | Depth 10' | Date 8/17/2021

Vertical Pressure (kips/sq.ft.)	Sample Height (inches)	Consolidation (percent)
0.125	**1.0000**	**0.0**
0.25	**0.9980**	**0.2**
0.5	**0.9960**	**0.4**
1	**0.9940**	**0.6**
1	**0.9480**	**5.2**
2	**0.9260**	**7.4**
4	**0.8920**	**10.8**
8	**0.8600**	**14.0**
0.25	**0.8790**	**12.1**

Saturated

Date Tested: **8/12/2021**
Sample: **T3**
Depth: **10'**



Silty Fine Sand w/ Clay
Dry Density: 92.8 pcf
Initial Moisture Content: 6.5 %
Saturated Moisture Content: 30.1 %
Saturated at 1 kip/sq.ft.

NorCal Engineering

SOILS AND GEOTECHNICAL CONSULTANTS

Tran Chung

PROJECT NUMBER: 22686-21 DATE: 8/17/2021

CONSOLIDATION TEST

ASTM D2435

Plate C

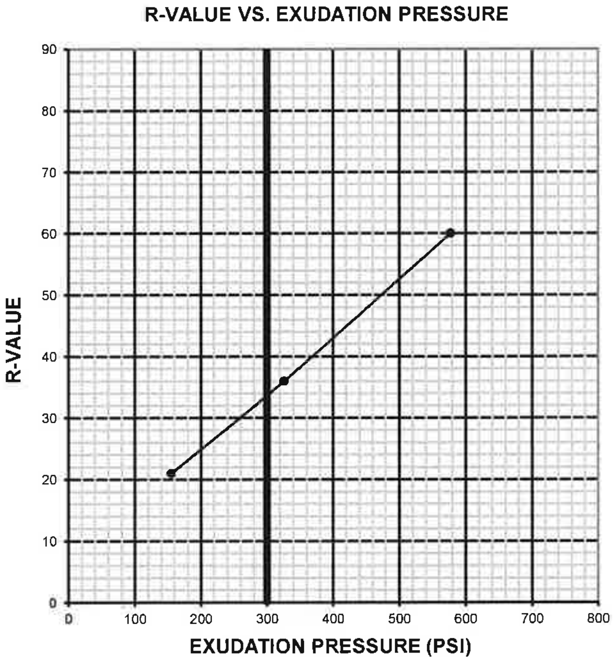
GMU

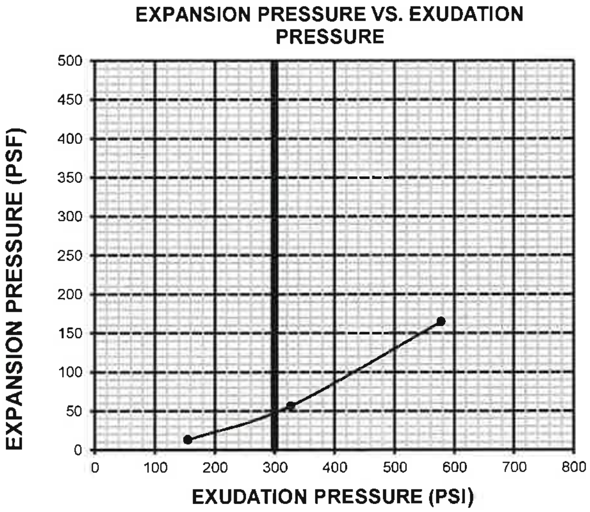
R-VALUE TEST REPORT

☑ CT-301 ☐ ASTM-D2844

PROJECT NAME: Norcal: Tran Cheng 22686-21
PROJECT NUMBER: L-210801
SAMPLE LOCATION: N of Alessandro Boulevard and E of Flaming Arrow Drive, Moreno Valley
SAMPLE NUMBER: T1
SAMPLE DESCRIPTION: CLAYEY SAND (SC), brown
SAMPLE DEPTH: 2'
SAMPLED BY: Norcal 7/23/21
TESTED BY: ER
DATE TESTED: 8/8/2021

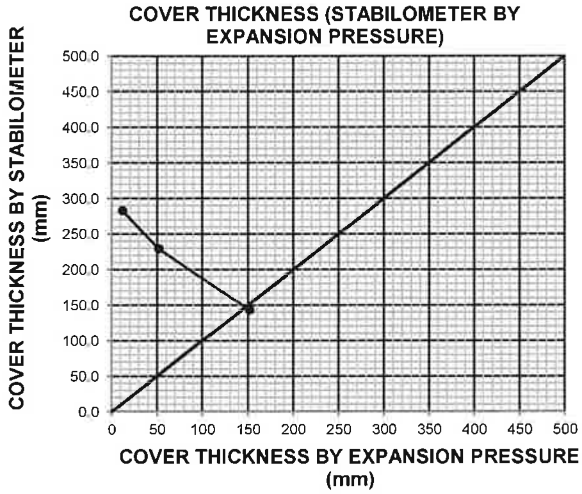
TEST SPECIMEN	A	B	C
MOISTURE AT COMPACTION %	8.7	9.7	10.7
WEIGHT OF SAMPLE, grams	1042	1086	1124
HEIGHT OF SAMPLE, Inches	2.33	2.35	2.48
DRY DENSITY, pcf	124.8	127.7	124.1
COMPACTOR AIR PRESSURE, psi	280	220	130
EXUDATION PRESSURE, psi	577	326	155
EXPANSION, Inches x 10exp-4	38	13	3
STABILITY Ph 2,000 lbs (160 psi)	40	72	100
TURNS DISPLACEMENT	4.07	4.68	5.80
R-VALUE UNCORRECTED	65	40	21
R-VALUE CORRECTED	60	36	21
EXPANSION PRESSURE (psf)	164.2	56.2	13.0





R-VALUE AT EQUILIBRIUM:	33

R-VALUE BY EXUDATION PRESSURE:	33
R-VALUE BY EXPANSION PRESSURE:	58
EXPANSION PRESSURE AT 300 PSI EXUDATION:	45
TRAFFIC INDEX (Assumed):	5.5
GRAVEL FACTOR (Assumed):	1.5
UNIT MASS OF COVER MATERIAL, kg/m^3 (Assumed):	2100.0



Appendix C
Seismic Hazard Report

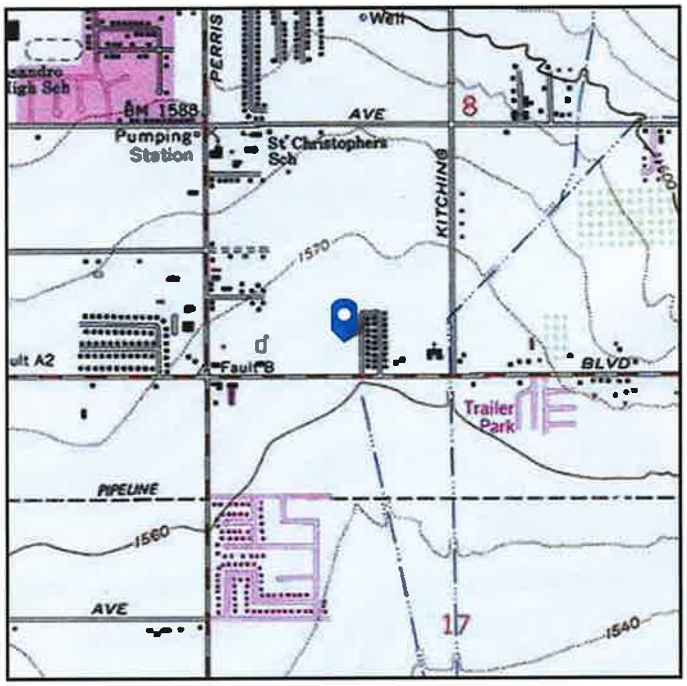
AMERICAN SOCIETY OF CIVIL ENGINEERS

ASCE 7 Hazards Report

Address:
No Address at This Location

Standard: ASCE/SEI 7-16
Risk Category: II
Soil Class: D - Stiff Soil

Elevation: 1566.66 ft (NAVD 88)
Latitude: 33.918062
Longitude: -117.221483



Seismic

Site Soil Class: D - Stiff Soil

Results:

S_S :	1.653	S_{D1} :	N/A
S_1 :	0.644	T_L :	8
F_a :	1	PGA :	0.7
F_v :	N/A	PGA_M :	0.77
S_{MS} :	1.653	F_{PGA} :	1.1
S_{M1} :	N/A	I_e :	1
S_{DS} :	1.102	C_v :	1.431

Ground motion hazard analysis may be required. See ASCE/SEI 7-16 Section 11.4.8.

Data Accessed: Wed Aug 25 2021

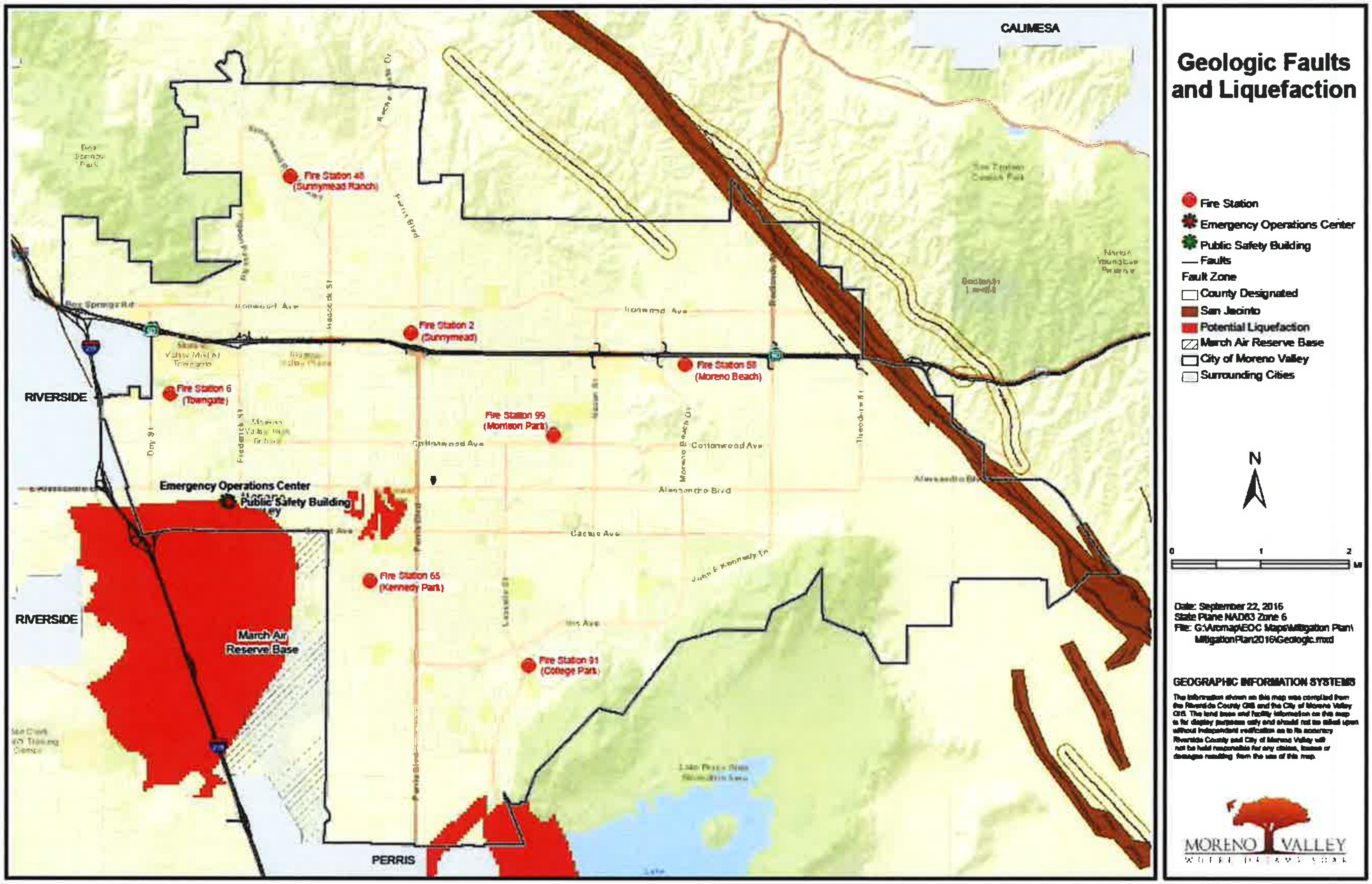
Date Source: USGS Seismic Design Maps

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.

Figure 4-1.1: Moreno Valley Geologic Faults and Liquefaction



Appendix D
Soil Infiltration Data

NORCAL ENGINEERING

SOILS AND GEOTECHNICAL CONSULTANTS

Project: Tran Chung
Project No.: 22686-21
Date: 7/23/2021
Test No. 1
Depth: 5’
Tested By: D.L.

TIME (hr/min)	CHANGE TIME (min)	CUMULATIVE TIME (min)	INNER RING READING (cm)	INNER RING CHANGE	INNER RING FLOW (cc)	OUTER RING READING (cm)	OUTER RING CHANGE	OUTER RING FLOW (cc)	INNER RING INF RATE (cm/hr)	OUTER RING INF RATE (cm/hr)	INNER RING INF RATE (ft/hr)
7:58			100.7			37.6					
8:13	15	15	101.6	0.9		38.1	0.5				
8:13			101.6			38.1					
8:28	15	30	102.6	1.0		38.6	0.5				
8:28			102.6			38.6					
8:43	15	45	103.6	1.0		39.3	0.7				
8:43			103.6			39.3					
8:58	15	60	104.4	0.8		40.0	0.7				
8:58			104.4			40.0					
9:13	15	75	105.1	0.7		40.6	0.6				
9:13			105.1			40.6					
9:28	15	90	105.9	0.8		41.1	0.5				
9:28			105.9			41.1					
9:43	15	105	106.6	0.7		41.7	0.6		2.8	2.4	
9:43			104.2			40.6					
9:58	15	120	104.6	0.4		41.1	0.5		1.6	2.0	
9:58			104.6			41.1					
10:13	15	135	104.9	0.3		41.5	0.4		1.2	1.6	
10:13			104.9			41.5					
10:28	15	150	105.2	0.3		41.9	0.4		1.2	1.6	
10:28			105.2			41.9					
10:43	15	165	105.4	0.2		42.3	0.4		0.8	1.6	
10:43			105.4			42.3					
10:58	15	180	105.2	0.3		42.9	0.6		1.2	2.4	

Average = 1.46 / 1.93 cm/hr

NORCAL ENGINEERING
SOILS AND GEOTECHNICAL CONSULTANTS

Project: Tran Chung	
Project No.: 22686-21	
Date: 7/23/2021	
Test No. 2	
Depth: 10'	
Tested By: D.L.	

TIME (hr/min)	CHANGE TIME (min)	CUMULATIVE TIME (min)	INNER RING READING (cm)	INNER RING CHANGE	INNER RING FLOW (cc)	OUTER RING READING (cm)	OUTER RING CHANGE	OUTER RING FLOW (cc)	INNER RING INF RATE (cm/hr)	OUTER RING INF RATE (cm/hr)	INNER RING INF RATE (ft/hr)
9:09			132.1			42.0					
9:24	15	15	132.5	0.4		43.0	1.0				
9:24			132.5			43.0					
9:39	15	30	133.0	0.5		43.8	0.8				
9:39			133.0			43.8					
9:54	15	45	133.6	0.6		44.7	0.9				
9:54			133.6			44.7					
10:09	15	60	134.2	0.6		45.5	0.8				
10:09			134.2			45.5					
10:24	15	75	134.4	0.2		46.1	0.6				
10:24			134.4			46.1					
10:39	15	90	137.7	0.3		46.8	0.7				
10:39			137.7			46.8					
10:54	15	105	138.1	0.4		47.5	0.7		1.6	2.8	
10:54			138.1			47.5					
11:09	15	120	138.5	0.4		48.1	0.6		1.6	2.4	
11:09			138.5			48.1					
11:24	15	135	138.8	0.3		48.8	0.7		1.2	2.8	
11:24			128.8			41.2					
11:39	15	150	129.1	0.3		42.1	0.9		1.2	3.6	
11:39			129.1			42.1					
11:54	15	165	129.6	0.6		42.9	0.8		2.4	3.2	
11:54			129.6			42.9					
12:09	15	180	130.0	0.4		43.5	0.6		1.6	2.4	

Average = 1.6 / 2.86 cm/hr